

Land of Cheese, Trees and Ocean Breeze

# NOTICE TO MORTGAGEE, LIENHOLDER, VENDOR OR SELLER: <br> ORS 215 REQUIRES THAT IF YOU RECEIVE THIS NOTICE, IT MUST BE PROMPTLY FORWARDED TO THE PURCHASER 

NOTICE OF PUBLIC HEARINGS TILLAMOOK COUNTY PLANNING COMMISSION "SECOND ADDITION TO AVALON HEIGHTS SUBDIVISION"

Date of Notice: May 17, 2021
Public hearings will be held by the Tillamook County Planning Commission at 6:30p.m. on Thursday, June 10th, 2021 and at $6: 30 \mathrm{pm}$ on Thursday, July 8th, 2021 in the Board of County Commissioners Meeting Rooms A \& B of the Tillamook County Courthouse, 201 Laurel Avenue, Tillamook, OR 97141 to consider the following:

851-21-000095-PLNG: A request for preliminary subdivision plat approval of a 58 -lot subdivision identified as "Second Addition to Avalon Heights" on a property located within the Unincorporated Community of Oceanside. The subject property is zoned Residential Oceanside (ROS) and is designated as Tax Lot 200 of Section 30DC, Township 1 South, Range 10 West of the Willamette Meridian, Tillamook County, Oregon. The Property Owner is Avalon Heights LLC. The Applicant is the Bill Hughes.

Notice of public hearing, a map of the request area, applicable specific request review criteria and a general explanation of the requirements for submission of testimony and the procedures for conduct of hearing has been mailed to all property owners within 250 feet of the exterior boundary of the subject property for which application has been made at least 10 days prior to the date of the hearing.

The applicable criteria include Tillamook County Land Division Ordinance (TCLDO) Section 070: Preliminary Plat Approval Criteria. Applicable development standards include TCLUO Section 3.310: Residential Oceanside Zone and TCLUO Section 4.130: Development requirements for Geologic Hazard areas. Only comments relevant to the approval criteria are considered relevant evidence.

Due to Governor Brown's Order limiting the number of persons allowed for public gatherings, the courthouse is not accessible to the public for these hearings. All hearings will take place virtually and will be livestreamed to ensure the public is able to participate. Oral testimony can be heard at the hearing on June 10, 2021. For instructions on how to provide oral testimony at the June 10, 2021 hearing, please visit the Tillamook County Community Development homepage at https://www.co.tillamook.or.us/commdev for instructions and protocol or email Allison Hinderer, DCD Office Specialist, at ahindere@co.tillamook.or.us. The livestream link will be provided at the DCD homepage address as well as a dial in number for those who wish to participate but are unable to participate virtually.

Written testimony may be submitted to the Tillamook County Department of Community Development, 1510-B Third Street, Tillamook, Oregon, 97141 prior to $4: 00$ p.m. on the date of the June 10, 2021 Planning Commission hearing. If submitted by 4:00 p.m. on June 1, 2021 the testimony will be included in the packet mailed to the Planning Commission the week prior to the June 10, 2021 hearing. Failure of an issue to be raised in a hearing, in person or by letter, or failure to provide sufficient specificity to afford the decision-maker an opportunity to respond to the issue precludes appeal to the Land Use Board of Appeals on that issue. Please contact Melissa Jenck, Project Planner, Tillamook County Department of Community Development, mjenck@co.tillamook.or.us as soon as possible if you wish to have your comments included in the staff report that will be presented to the Planning Commission.

The documents and submitted application are also available on the Tillamook County Department of Community Development website (https://www.co.tillamook.or.us/commdev/landuseapps) or at the Department of Community Development office located at 1510-B Third Street, Tillamook, Oregon 97141. A copy of the application and related materials may be purchased from the Department of Community Development at a cost of 25 cents per page. The staff report will be available for public inspection on June 3, 2021. Please contact Allison Hinderer for additional information ahindere@co.tillamook.or.us or call 1-800-488-8280 ext. 3423.

In addition to the specific applicable review criteria, the Tillamook County Land Use Ordinance, Tillamook County Land Division Ordinance, Tillamook County Comprehensive Plan, and Statewide Planning Goals which may contain additional regulations, policies, zones and standards that may apply to the request are also available for review at the Department of Community Development.

The Tillamook County Courthouse is handicapped accessible. If special accommodations are needed for persons with hearing, visual, or manual impairments who wish to participate in the hearing, please contact 1-800-488-8280 ext. 3303, at least 24 hours prior to the hearing in order that appropriate communications assistance can be arranged.

If you need additional information, please contact Allison Hinderer, DCD Office Specialist, at 1-800-488-8280 ext. 3423 or email ahindere@co.tillamook.or.us.

Sincerely,
Tillamook County Department of Community Development


Sarah Absher, CFM, Director

Enc. Applicable Ordinance Criteria (Already in Record) Maps (Already in Record)
Second Addition to Avalon Heights Subdivision Preliminary Plat (Already in Record)
Tips for Citizen Testimony (Already in Record)
Procedures for conduct at a public hearing (Already in Record)

# REVIEW CRITERIA \& DEVELOPMENT STANDARDS 

TILLAMOOK COUNTY LAND USE ORDINANCE<br>https://www.co.tillamook.or.us/commdev/page/land-use-ordinance-luo-zoning-ordinance

TCLUO Section 3.310: Residential Oceanside (ROS) Zone: TCLUO Section 3.310(4)
(a) The minimum lot size for permitted uses shall be 7,500 square feet where the slope averages less than 19 percent. Where the slope averages from 19 to 29 percent the minimum lot size shall be 10,000 square feet, and where the slope averages greater than 29 percent, the minimum lot size shall be 20,000 square feet, except that in both of theses sloped areas and in unsewered or geologic hazard areas, a larger minimum may be required. [Refer to Article V Exceptions for existing legally platted lots and parcels]
(b) The minimum lot width shall be 60 feet.
(c) The minimum lot depth shall be 75 feet.
(j) A property survey of the lot shall be performed including elevations, and all corners shall be monumented by a registered surveyor prior to land division and/or submittal of a permit for construction/location on lots containing less than 7,500 square feet. A copy of the survey shall be submitted with the application and other required material.
(l) New uses authorized within the community growth boundary shall not adversely affect farm or forest management practices conducted in accordance with federal and state laws. Authorization to create a parcel or construct a dwelling adjacent to land zoned for farm or forest use shall require a notarized declaratory statement signed by all current property owners who appear on the property deed or contract. This statement shall serve as a covenant that runs with the land binding heirs, assigns, lessees and successors. This covenant shall affirm that residents of the parcel may be subject to farm or forest management practices conducted in accordance with federal and state laws which ordinarily and necessarily produce noise, dust, smoke and other impacts. Those signing the statement acknowledge that they "do hereby accept the potential impacts from farm and forest practices as normal and necessary and part of the risk of establishing a dwelling in this area, and acknowledge the need to avoid activities that conflict with nearby farm or forest uses". The signed and notarized covenant must be approved by the County Planning Director and recorded with the Tillamook County Clerk.

TCLUO Section 4.130: Development Requirements for Geologic Hazard Areas: TCLUO Section 4.130 subsections (2) through (8) must be met.

## TILLAMOOK COUNTY LAND DIVISION ORDINANCE

https://www.co.tillamook.or.us/sites/default/files/fileattachments/community_development/page/27173/final_land_di vision_ordinance.pdf

Section 060: Preliminary Plat Submission Requirements, including Section 060(1)(a), Section 060(1)(b)(i)(1) through (7), Section 060(1)(b)(ii)(1) through (8), (10) and (11), Section 060(1)(b)(iii)(1) and (3) through (12), Section 060(1)(c)(ii) through (v), (vii) and (viii). And Section 060(1)(d).

Section 070: Preliminary Plat Approval Criteria, specifically Section 070(1)(a) through (i) and Section 070(2).
Section 150: Development Standards for Land Divisions, including Section 150(1) through (5), (7), (10) and (11).
Section 160: Street Improvements, including Section 160(1) through (3), (5), (7) and (8).


## Tillamook County GIS

Vicinity Map: 851-21-000095-PLNG Subdivision Second Addition to Avalon Heights


Tillamook County GIS
Zoning Map: 851-21-000095-PLNG Subdivision Second Addition to Avalon Heights


Created: Mon May 17 2021-12:25:19
Active Layers:County_Boundary, Fed_state_highways, citylimit, community_polygon, TaxlotOwner, highlight, Tillamook_County_Zoning, Township_Range_Section, Road_Centerline
Extent:-13800512.608554, 5691734.3390818, -13796654.927284, 5693704.981217


# TILLAMOOK County Assessor's Summary Report <br> Real Property Assessment Report 

FOR ASSESSMENT YEAR 2020
May 17, 2021 12:27:53 pm


Comments: $\quad$ 2/27/12 Land reappraisal, tabled land, size change per cartographer, split FPNW w/U2.LM 8/1/17 - Updated FP values after PA conversion - changed to entered values. EJ.
02/22/18 Combined U2 account into U1 account. U1 account was retaxlotted into TL 200. Canceled U2 account.ef 06/11/19 Changed land back to trendable.ef

National Flood Hazard Layer FIRMette
$123^{\circ} 57^{\prime} 47^{\prime \prime} \mathrm{W} 45^{\circ} 27^{\prime} 19^{\circ} \mathrm{N}$


## Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

|  | Without Base Flood Elevation (BFE) <br> Zone A $V$, As9 |
| :--- | :--- |
| With BFE or Depth Zone AE, AO, AH, VE, AR |  |
| SPECIAL FLOOD |  |
| HAZARD AREAS |  |$\quad$| Regulatory Floodway |
| :--- |

B $\mathbf{2 0 . 2}$ Cross Sections with 1\% Annual Chance
17.5 Water Surface Elevation

8- - - Coastal Transect
m $510=$. Base Flood Elevation Line (BFE)
工 Limit of Study

- Jurisdiction Boundary
--- --- Coastal Transect Baseline
OTHER FEATURES $\qquad$ Profile Baseline
Hydrographic Feature
MAP PANELS
$\square$ Digital Data Available
$\square$ No Digital Data Available (Z) Unmapped
$\overbrace{1}^{N}$

The pin displayed on the map is an approximate point selected by the user and does not represe an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards
The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on $5 / 17 / 2021$ at $3: 31$ PM and does not time The NFH and effective information may change and change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels elements do not appear: basemap imagery, flood zone labels,
legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.


May 17, 2021

Wetlands
Estuarine and Marine DeepwaterEstuarine and Marine Wetland

Freshwater Emergent Wetland Freshwater Forested/Shrub Wetland

This map is for general reference only. The US Fish and Wildlife Service is not responsible for the accuracy or currentness of the base data shown on this map. All wetlands related data should be used in accordance with the layer metadata found on the
Wetlands Mapper web site.



Tillamook County Department of Community Development

## LAND DIVISION APPLICATION

## Applicant $\square$ (Check Box if Same as Property Owner)

| Name: Bill Hughes | Phone: | $503-741-6706$ |
| :--- | :--- | :--- |
| Address:41901 Old Highway 30 |  |  |
| City: Astoria | State:OR | Zip: 97103 |
| Email:bchexc@ gmail.com |  |  |
| Property Owner | Phone: $(503) 741-6706$ |  |
| Name: Avalon Heights LLC | State:OR | Zip:97103 |
| Address: 41901 Old Highway 30 |  |  |
| City: Astoria |  |  |

Fax: 503-842-1819


Site Address: not yet assigned
Map Number: T1S R10W 30DC TL 200
Township Range Tax Lot(s)

Land Division Type: $\square$ Partition (Two or Three Lots, Type II) ■Subdivision (Four or More Lots, Type III)
Preliminary Plat (Pages 1-2)

## ■ PRELIMINARY PLAT (LDO 060(1)(B))

E For subdivisions, the proposed name.

- Date, north arrow, scale of drawing.
- Location of the development sufficient to development sufficient to define its location, boundaries, and a legal description of the site.
- Existing streets with names, right-ofway, pavement widths, access points.
- Width, location and purpose of existing easements
E The location and present use of all structures, and indication of any that will remain after platting.
E Location and identity of all utilities on and abutting the site. If water mains and sewers are not on site, show distance to the nearest one and how they will be brought to standards
E Location of all existing subsurface sewerage systems, including drainfields and associated easements


## General Information

E Parcel zoning and overlays
E Title Block

- Clear identification of the drawing as "Preliminary Plat" and date of preparation
E Name and addresses of owner(s), developer, and engineer or surveyor


## Existing Conditions

E Ground elevations shown by contour lines at 2 -foot vertical interval. Such ground elevations shall be related to some established benchmark or other datum approved by the County Surveyor

- The location and elevation of the closest benchmark(s) within or adjacent to the site
E Natural features such as drainage ways, rock outcroppings, aquifer recharge areas, wetlands, marshes, beaches, dunes and tide flats
트 For any plat that is 5 acres or larger, the Base Flood Elevation, per FEMA Flood Insurance Rate Maps
Proposed Development

```
E Proposed lots, streets, tracts,
    open space and park land (if any);
    location, names, right-of-way
    dimensions, approximate radius of
    street curves; and approximate
    finished street center line grades.
    All streets and tracts that are
    being held for private use and all
    reservations and restrictions
    relating to private tracts identified
@ Location, width and purpose of all
    proposed easements
```

```
        Proposed deed restrictions, if any,
        in outline form
E Approximate dimensions, area
    calculation (in square feet), and
    identification numbers for all
    proposed lots and tracts
- Proposed lots, streets, tracts, open space and park land (if any); location, names, right-of-way dimensions, approximate radius of street curves; and approximate finished street center line grades. All streets and tracts that are being held for private use and all reservations and restrictions relating to private tracts identified
- Location, width and purpose of all proposed easements
Proposed deed restrictions, if any, in outline form
- Approximate dimensions, area calculation (in square feet), and identification numbers for all proposed lots and tracts
```

Proposed Development

- Proposed uses of the property, including all areas proposed to be dedicated as public right-of-way or reserved as open space
E On slopes exceeding an average grade of $10 \%$, as shown on a submitted topographic survey, the preliminary location of development on lots demonstrating that future development can meet minimum required setbacks and applicable engineering design standards
E Preliminary utility plans for sewer, water and storm drainage when these utilities are to be provided
- The approximate location and identity of other utilities, including the locations of street lighting fixtures, as applicable
트․ Evidence of compliance with applicable overlay zones, including but not limited to the Flood Hazard Overlay (FH) zone
트 Evidence of contact with the applicable road authority for proposed new street connections
- Certificates or letters from utility companies or districts stating that they are capable of providing service to the proposed development


## Additional Information Required for Subdivisions

E Preliminary street layout of undivided portion of lot
E Special studies of areas which appear to be hazardous due to local geologic conditions
틍 Where the plat includes natural features subject to the conditions or requirements contained in the County's Land Use Ordinance, materials shall be provided to demonstrate that those conditions and/or requirements can be met
— Approximate center line profiles of streets, including extensions for a reasonable distance beyond the limits of the proposed Subdivision, showing the proposed finished grades and the nature and extent of construction

Profiles of proposed drainage waysIn areas subject to flooding, materials shall be submitted to demonstrate that the requirements of the Flood Hazard Overlay (FHO) zone of the County's Land Use Ordinance will be met
■ If lot areas are to be graded, a plan showing the nature of cuts and fills, and information on the character of the soilProposed method of financing the construction of common improvements such as street, drainage ways, sewer lines and water supply linesFINAL PLAT (LDO 090(1))Date, scale, north arrow, legend, highways, and railroads contiguous to the plat perimeterDescription of the plat perimeterThe names and signatures of all interest holders in the land being platted, and the surveyor Monuments of existing surveys identified, related to the plat by distances and bearings, and referenced to a document of recordExact location and width of all streets, pedestrian ways, easements, and any other rights-of-wayEasements shall be denoted by fine dotted lines, and clearly identified as to their purposeProvisions for access to and maintenance of off-right-of-way drainageBlock and lot boundary lines, their bearings and lengthsBlock numbersLot numbersThe area, to the nearest hundredth of an acre, of each lot which is larger than one acreIdentification of land parcels to be dedicated for any purpose, public or private, so as to be distinguishable from lots intended for sale

## Authorization

This permit application does not assure permit approval. The applicant and/or property owner shall be responsible for obtaining any other necessary federal, state, and local permits. Within two (2) years of final review and approval, all final plats for land divisions shall be filed and recorded with the County Clerk, except as required otherwise for the filing of a plat to lawfully establish an unlawfully created unit of land. The applicant verifies that the information submitted is complete, accurate, and consistent with other information submitted with this application.


| From: | BLAIR Keith P [Keith.P.BLAIR@odot.state.or.us](mailto:Keith.P.BLAIR@odot.state.or.us) |
| :--- | :--- |
| Sent: | Friday, July 10, 2020 8:29 AM |
| To: | Melissa Webb; STRAUSS Karen A |
| Cc: | Erik Hoovestol; Bill Hughes |
| Subject: | RE: Draft - TIS for Second Avalon Heights Subdivision (Oceanside) |

Karen and Melissa:
I have reviewed the draft traffic study and have one minor comment which I recommend be incorporated within a final version to be submitted with the application to the County:

- Page 18, "Performance Standards" section - The as the study intersection is located outside any urban growth boundary and within "Unincorporated Communities," the mobility target is actually 0.80 rather than 0.95 .

Please let me know if you have any questions or if I may be of further assistance. Thanks!
Keith P. Blair, P.E.
Region Traffic Manager | ODOT Region 2
455 Airport Rd SE, Bldg. A | Salem, Oregon 97301
(503) 986-2656 | Keith. P. Blair@odot.state.or.us

ODOT's mission is to provide a safe and reliable multimodal transportation system that connects people and helps Oregon's communities and economy thrive.

From: Melissa Webb [melissa@lancastermobley.com](mailto:melissa@lancastermobley.com)
Sent: Thursday, July 9, 2020 2:10 PM
To: BLAIR Keith P [Keith.P.BLAIR@odot.state.or.us](mailto:Keith.P.BLAIR@odot.state.or.us); STRAUSS Karen A [Karen.A.STRAUSS@odot.state.or.us](mailto:Karen.A.STRAUSS@odot.state.or.us)
Cc: Erik Hoovestol [eh@firwooddesign.com](mailto:eh@firwooddesign.com); Bill Hughes [bchexc@gmail.com](mailto:bchexc@gmail.com)
Subject: Draft - TIS for Second Avalon Heights Subdivision (Oceanside)
Hi Keith and Karen.
Per our conversation this morning. attached is a draft report of the traffic study for the Second Avalon Heights subdivision in Oceanside. Any comments you have that I can address/incorporate in the study prior to submission to the County would be appreciated.

Thank you again for your willingness to review the draft copy.
Melissa
--
Melissa Webb, PE
Transportation Analysi
 Wifi

## Preliminary Plat Application Narrative for <br> Second Addition to Avalon Heights

Mr. Bill Hughes as a member of Avalon Heights LLC has applied for preliminary plat approval for a 58 -lot subdivision for the ultimate construction of single-family dwellings in the unincorporated community of Oceanside. The subject property is identified as Tax Lot 200 on Taxmap 01S10W30DC. The project would be developed in three phases. The property is zoned Residential Oceanside and a portion of the eastern side of the property is within a Geologic Hazard Overlay Zone.

The minimum lot size is 7,500 square feet for lots with average slopes less than 19 percent; 10,000 square feet where average slopes are between 19 percent and 29 percent; and 20,000 square feet where average slopes exceed 29 percent. See the notes on application Drawing Set Sheet 3 and the noted slopes and direction of fall on Sheet 6 . The minimum lot width is 60 feet and minimum depth is 60 feet. Front yard setback is 20 feet; side yard setbacks are generally 5 feet but 15 feet on corner lots; and the minimum rear yard set back is generally 15 feet, but 5 feet on corner lots. Setback lines are illustrated on Sheet 6 together with conceptual building footprints.

## Submission Requirements (Section 060)

(1) Applications for Preliminary Plat approval shall contain the following information:

Preliminary Plat Information. In addition to the general information described in Subsection (a) above, the Preliminary Plat application shall consist of drawings and supplementary material adequate to provide the following information, in quantities determined by the County Surveyor and Tillamook County Planning Commission.
I. General Information.
1.For subdivisions, the proposed name shall not duplicate or resemble the name of another land division in the County, and shall be approved by the County Surveyor.
2.Date, north arrow, scale of drawing.
3.Location of the development sufficient to define its location, boundaries, and a legal description of the site.
4.Zoning of parcel to be divided, including any overlay zones.
5.A title block including the names, addresses, and telephone numbers of the owners of the subject property and, as applicable, the name of the engineer and surveyor, and the date of the survey.
6.Clear identification of the drawing as a "Preliminary Plat" and date of preparation.
7.Name and addresses of the owner(s), developer, and the engineer or surveyor.

Applicant response: All of this information is included on one or more of the drawing set sheets as appropriate.
ii. Existing Conditions. Except where the Director deems certain information is not relevant, applications for Preliminary Plat approval shall contain all of the following information on existing conditions:
1.Existing streets or roads (public or private), including location, names, right-of-way and pavement widths on and abutting the site; and location of existing access point

Applicant Response: See Sheet 4
2.Width, location and purpose of all existing easements of record on and abutting the site;

## Applicant Response: See Sheet 2.

3.The location and present use of all structures on the site and indication of which, if any structures are to remain after platting;

Applicant Response: See Sheet 2.
4.Location and identity of all utilities on and abutting the site. If water mains and sewers are not on or abutting the site, indicate the direction and distance to the nearest one and show how utilities will be brought to standards;

## Applicant Response: See Sheet 4.

5.Location of all existing subsurface sewerage systems, including drain fields and associated easements on the site.

Applicant Response: There are no subsurface sewerage systems on site.
6.Ground elevations shown by contour lines at 2-foot vertical interval. Such ground elevations shall be related to some established benchmark or other datum approved by the County Surveyor; the Director may waive this standard for partitions when grades, on average, are less than 10 percent;

> Second Addition to Avalon Heights
> Preliminary Plat Application Narrative Page 2
7.The location and elevation of the closest benchmark(s) within or adjacent to the site (i.e., for surveying purposes);

Applicant Response: See Sheet 7. The benchmark is located in the southeasterly corner of the parcel.
8.Natural features such as drainage ways, rock outcroppings, aquifer recharge areas, wetlands, marshes, beaches, dunes and tide flats;

Applicant Response: See Sheet 2. There are no rock outcroppings, aquifer recharge areas, wetlands, marshes, beaches, dunes and tide flats on the site.
9.Any plat that is five (5) acres or larger, or proposes 50 lots or greater, shall include the Base Flood Elevation, per FEMA Flood Insurance Rate Maps,

The closest 100-year flood plain per FEMA is located on the coast of the Pacific Ocean with a Base Flood Elevation of 26.5 feet. The site is located approximately 0.3 miles from the coast with the lowest elevation of approximately 290 feet.
10.North arrow and scale; and

Applicant Response: North arrows and scales are included on each drawing of the plan set except the cover sheet.
11.Other information, as deemed necessary by the Planning Director for review of the application. The County may require studies or exhibits prepared by qualified professionals to address specific site features and code requirements.

Applicant Response: A Traffic Impact Analysis, Geohazard Analysis and a Preliminary Stormwater Plan are included in the application package.
iii. Proposed Development. Except where the Director deems certain information is not relevant, applications for Preliminary Plat approval shall contain all of the following information on the proposed development:
1.Proposed lots, streets, tracts, open space and park land (if any); location, names, right-of-way dimensions, approximate radius of street curves; and approximate finished street center line grades. All streets and tracts that are being held for private use and all reservations and restrictions relating to such private tracts shall be identified;

> Second Addition to Avalon Heights
> Preliminary Plat Application Narrative Page 3
2.City boundary lines when crossing or adjoining the subdivision;

Applicant Response: Not applicable.
3.Easements: location, width and purpose of all proposed easements;

Applicant Response: See Sheet 4.
4.Proposed deed restrictions, if any, in outline form.

Applicant Response: A home owners association or road maintenance district will be formed to finance stormwater facility maintenance and road maintenance.
5.Lots and private tracts (e.g., private open space, common area, or street): approximate dimensions, area calculation (e.g., in square feet), and identification numbers for all proposed lots and tracts;

## Applicant Response: See Sheet 4.

6. Proposed uses of the property, including all areas proposed to be dedicated as public right-ofway or reserved as open space for the purpose of surface water management, recreation, or other use;

Applicant Response: A stormwater infiltration pond will be located in an easement on Lots 1 through 7. Another easement for stormwater conveyance via a swale will be located on Lots 29 through 32 and Lot 38. A storm and water easement to the Netarts Water District is located on Lot 24. An access easement is located on the back of lots $88,9,10$,, and Lot 58 to encompass an existing roadway which falls outside of the existing easement.
7.On slopes exceeding an average grade of $10 \%$, as shown on a submitted topographic survey, the preliminary location of development on lots (e.g., building envelopes), demonstrating that future development can meet minimum required setbacks and applicable engineering design standards;

## Applicant Response: See Sheet 6.

8.Preliminary utility plans for sewer, water and storm drainage when these utilities are to be provided. This information may be included on the preliminary plat map provided all information is legible.

> Second Addition to Avalon Heights
> Preliminary Plat Application Narrative Page 4

## Applicant Response: See Sheet 4.

9.The approximate location and identity of other utilities, including the locations of street lighting fixtures, as applicable;

Applicant Response: See Sheet 4.
10.Evidence of compliance with applicable overlay zones, including but not limited to the Flood Hazard Overlay (FH) zone;

Applicant Response: A geologic hazard analysis is attached to the application in compliance with the standards governing the geo-hazard overlay. A completed geotechnical report for the development is currently in progress and will be provided to the County with the construction permit plans.
11.Evidence of contact with the applicable road authority for proposed new street connections; and

The proposed development connects to NW Highland Drive which is a local road not maintained by the County but is within a County owned right-of-way. ODOT has reviewed the Traffic Impact Study and concurred that no improvements were warranted at the intersection of Highland Drive and ODOT Highway 131.
12.Certificates or letters from utility companies or districts stating that they are capable of providing service to the proposed development.

Applicant Response: See attached letter from the Netarts-Oceanside Sanitary District and certificate from the Netarts Water District.

## SECOND ADDITION TO AVALON HEIGHTS SUBDIVISION

## LAND USE PLANS

TAXMAP: 01S10W30DC TAXLOT: 200 LOCATED IN SE $1 / 4$ OF SEC 30 T1S R10W W.M.

TILLAMOOK COUNTY, OREGON


SHEET INDEX:
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7 - Roaring tide loop concept profle
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9 - nw ocean song concept profle
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3 - Concept inflitration pond sections
14 - CUT-FILL MAP


VERTICAL DATUM: NAVD 88
COORDINATE SYSTEM: OREGON COAST ZONE

 41901 OLD HIGHWAY 30














## PRELIMINARY STORMWATER REPORT

Second Avalon Heights Subdivision
Located in SE $1 / 4$ of SE $1 / 4$ of Sec 30 T1S R10W W.M.
Taxlot 200
Tillamook County, OR
March 31, 2021

Prepared By:

Preliminary Stormwater Report for Land Use Submittal

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III. RUNOFF, CONVEYANCE, AND INFILTRATION

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HydraFlow Express Output
B. Referenced Data

USDA Web Soil Survey Map
ODOT Manning's Values Tables
Stormwater Infiltration Test by EMS Inc.
C. Basin Map

## I. PROJECT DESCRIPTION \& OBJECTIVE

The proposed project is a single-family residential subdivision encompassing approximately 21 acres of unincorporated Tillamook County near Oceanside. The project location is shown on the map below.


Currently, much of the project site drains to a large existing on-site depression or basin located at the south west corner of the site where runoff infiltrates into native soils. Postdevelopment, the subdivision will drain to and be infiltrated in the same location. The objective of this preliminary stormwater report is to demonstrate feasibility of the conceptual stormwater management plan for the land use phase of this project. Final detailed design and plans will be provided for construction permitting.
II. SITE DATA

## Site Rainfall Data

Rainfall data for the site was obtained from the NOAA Atlas 2 Precipitation Frequency Estimate tool: NOAA Atlas 2 Precipitation Frequency Estimates (weather.gov)

## Precipitation Frequency Data Output

| Map | Precipitation (inches) | Precipitation Intensity (in/hr) |
| :---: | :---: | :---: |
| 2-year 6-hour | 1.51 | 0.25 |
| 2-year 24-hour | 3.05 | 0.13 |
| 100-year 6-hour | 3.00 | 0.50 |
| 100-year 24-hour | 6.50 | 0.27 |

## Go to PFDS <br> Go to NA. 2

Hydrometeorological Design Studies Center - NOAA/National Weather Service
1325 East-West Highway - Silver Spring, MD 20910 - (301) 713-1669
Mon Jan 11 17:23:53 2021
The 6.5 -inch 100 -year 24 -hour design storm will be used for this project.

## Site Soils

Soil data for the site was obtained from the U.S. Department of Agriculture (USDA) Natural Resources Conservation Service (NRCS) Web Soil Survey. The soil resource report is included in the appendix for reference. The site, especially areas tributary to the infiltration area, consists primarily of Netarts fine sandy loam, a Type A hydrologic group soil with a Ksat (capacity of most limiting layer to transmit water) of 1.98 to 5.95 inches per hour.

Environmental Management Systems Inc (EMS) performed an infiltration test in the approximate location of the proposed infiltration pond. The report is included in the appendix for reference. Two falling-head infiltration tests were performed; the second infiltration test result (19.5 inches per hour) was slightly lower than the first test and is therefore used for design. A safety factor of two is applied, so the design infiltration rate used is 9.75 inches per hour.

## III. RUNOFF, CONVEYANCE, AND INFILTRATION

## Runoff

A basin map and HydroCAD model were developed for concept-level hydrological and infiltration calculations; both are included in the appendix for reference. As part of developing the basin map, assumptions were made for impervious surfaces at full buildout; lots are assumed to average approximately 4,000 square feet of impervious per lot, which equates to a 50 ' $\times 50$ ' house and a 75 ' $\times 20^{\prime}$ driveway. Roadway impervious areas were calculated from the concept design drawings.

The HydroCAD model uses the Santa Barbara Urban Hydrograph (SBUH) with a Type 1A rainfall distribution methodology. The time of concentration for the basin was calculated using the basin map and the HydroCAD time of concentration calculation tool for the most hydraulically distant point of the drainage basin.

The Curve Numbers (CN) used in hydrological calculations are:
Impervious (pavement, gravel, driveways, and houses): 98
On-site pervious (lawns, roadside ditches, stormwater easement, Type A soil): 49
Off-site pervious (woods): 36
Calculated peak runoff rates from the 100-year, 24-hour design storm entering the infiltration basin is tabulated below.

| Basin 1 | Basin 2 | Total |
| :---: | :---: | :---: |
| 6.34 cfs | 2.99 cfs | 9.33 cfs |

## Conveyance

The capacity of roadside ditches and culverts was calculated with Manning's Equation using HydraFlow Express, an extension for AutoCAD Civil3D. Manning's coefficients used are from the ODOT Hydraulics Manual, Chapter 8, Appendix A - Hydraulic Roughness (Manning's n) Values of Conduits and Channels. The HydraFlow calculations and an excerpt of the ODOT tables are included in the appendix of this report.

Maximum capacity of stormwater conveyance facilities:
Roadside ditch with 0 " freeboard at $1.00 \%$ slope: 13.79 cfs
18 " smooth plastic at $1.00 \%$ slope: 13.35 cfs
12 " smooth plastic at $1.00 \%$ slope: 4.53 cfs
12 " smooth plastic at $2.00 \%$ slope: 6.40 cfs
As the minimum proposed road grade is $1 \%$, roadside ditch and pipe capacity at $1 \%$ slope was checked against the peak runoff flow rates from the 100-year design storm for Basin 1. As shown, all roadside ditches and 18 " smooth plastic storm lines have sufficient capacity to convey peak flow rates. At $1 \%$ minimum grade, 12 " smooth plastic storm lines do not have sufficient capacity to convey the peak flow rate; the minimum slope required for capacity was calculated to be $2.00 \%$. As most of the proposed roadway grade

## Preliminary Stormwater Report for Land Use Submittal

is steeper than this minimum, most individual lot driveway culverts can be 12 " diameter. On any driveway approaches where conveyance capacity cannot be met with 12 " diameter culverts, an $18^{\prime \prime}$ culvert may be installed. Therefore, the concept design of roadside ditches and culverts is feasible.

## Infiltration

Currently, much of the project property drains to an existing on-site low point. After development, most of the project property and some off-site areas will drain to this low point. As proposed development will create a significant amount of impervious surface, the existing infiltration location will have to be enlarged. As discussed in Section II of the report, the design infiltration rate for this basin is 9.75 inches per hour.

The basin is situated at the bottom of a large hill; its geometry is designed to roughly fit the existing hill topography. The concept basin was sized using stage storage with HydroCAD and AutoCAD Civil3D modeling. Tributary runoff hydrograph, basin stage storage volume, and exfiltration from the basin was calculated/modeled using HydroCAD. Refer to the concept infiltration pond plan for additional information on the configuration of the pond and maintenance access road.

Additional considerations to be addressed with final design:

- Lots adjacent to the infiltration basin should have a building finish floor elevation above the infiltration basin emergency overflow elevation.
- Basin side-slopes, especially portions located along the existing hill, may need stabilization measures such as riprap, erosion control blankets, or vegetation.
- Erosion protection and sediment/trash capture to protect the basin from erosion and excessive sedimentation, such as a forebay, riprap protection, or other design measures, may be required.
- The size of the drainage basin tributary to the infiltration basin, and therefore the size of the infiltration basin, may be reduced by utilizing strategically placed drywells. This alternative design approach will considered during the value engineering phase of this project.
- Infiltration of stormwater in the roadside ditches is not analyzed separately in the preliminary design for two reasons. First, infiltration of rainfall in the ditches is generally accounted for by including the ditches in the Curve Number calculations as pervious area. Second, significant lengths of the roadside ditches will be piped at full build-out by driveway culverts, reducing the length of ditch where any additional infiltration may occur. Final design of roadside ditch may include rock check dams or other facilities to increase hydraulic residence time and therefore increased infiltration in roadside ditches.


## APPENDIX A

Calculations


## Basin 2



Time span=0.00-36.00 hrs, dt=0.01 hrs, 3601 points
Runoff by SBUH method, Split Pervious/Imperv.
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

## Subcatchment 1S: Basin 1

Subcatchment 2S: Basin 2

Pond 1P: Pond

Total Runoff Area $=18.825$ ac Runoff Volume $=1.573$ af Average Runoff Depth $=1.00$ "
$\mathbf{6 5 . 1 2 \%}$ Pervious $=12.259$ ac $34.88 \%$ Impervious $=6.566$ ac

## Summary for Subcatchment 1S: Basin 1

Runoff $=2.87$ cfs @ 8.00 hrs, Volume $=1.086$ af, Depth= $0.89{ }^{\prime \prime}$

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-36.00 hrs, dt= 0.01 hrs Type IA 24-hr 2-Yr Rainfall=3.05"

|  | ea (sf) | CN | escription |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 62,500 | 98 | Roads \& Shared Driver |  |  |
|  | 36,000 | 98 | On-Site Houses \& Driveways (34) |  |  |
|  | 5,500 | 49 P | Pervious - 50-75\% Grass cover, Fair, HSG A |  |  |
|  | 56,000 | 36 | Offsite - Woods, Fair, HSG A |  |  |
| 640,000 |  | 61 | Weighted Average |  |  |
| 441,500 |  | 44 | 68.98\% Pervious Area |  |  |
| 198,500 |  | 98 | 31.02\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 7.6 | 110 | 0.3500 | 0.24 |  | Sheet Flow, Sheet - Offsite Woods |
|  |  |  |  |  | Woods: Light underbrush $\mathrm{n}=0.400 \mathrm{P} 2=3.05{ }^{\prime \prime}$ |
| 3.8 | 300 | 0.0700 | 1.32 |  | Shallow Concentrated Flow, Offsite Woods |
|  |  |  |  |  | Woodland Kv=5.0 fps |
| 4.0 | 1,470 | 0.0720 | 6.06 | 48.46 | Channel Flow, Swale/Ditch |
|  |  |  |  |  | Area $=8.0 \mathrm{sf}$ Perim=26.0' r= 0.31' |
|  |  |  |  |  | $\mathrm{n}=0.030$ Earth, grassed \& winding |

15.4 1,880 Total

## Subcatchment 1S: Basin 1



## Summary for Subcatchment 2S: Basin 2

Runoff $=1.28 \mathrm{cfs} @ 8.00 \mathrm{hrs}$, Volume= 0.486 af, Depth= $1.41^{\prime \prime}$
Runoff by SBUH method, Split Pervious/Imperv., Time Span= $0.00-36.00 \mathrm{hrs}$, dt= 0.01 hrs Type IA 24-hr 2-Yr Rainfall=3.05"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| * | 15,500 | 98 | On-Site Roads |  |  |
| * | 44,000 | 98 | On-Site Houses \& Driveways (11) |  |  |
| * | 20,000 | 98 | Off-Site Houses \& Driveways |  |  |
| * | 92,500 | 49 P | Pervious -50-75\% Grass cover, Fair, HSG A |  |  |
| * | 8,000 | 98 | Off-Site - Future Highland Road |  |  |
| 180,000 |  |  | Weighted Average |  |  |
|  | 92,500 | 49 | 51.39\% Pervious Area |  |  |
| 87,500 |  | 98 | 48.61\% Impervious Are |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope $(\mathrm{ft} / \mathrm{ft})$ | $\begin{array}{r} \text { Velocity } \\ \text { (ft/sec) } \\ \hline \end{array}$ | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 13.3 | 280 | 0.2000 | 0.35 |  | Sheet Flow, Sheet - Lot Yards |
|  |  |  |  |  | Grass: Dense n=0.240 P2=3.05" |
| 1.2 | 480 | 0.0900 | - 6.77 | 54.18 | Channel Flow, Future Road Ditch |
|  |  |  |  |  | Area $=8.0$ sf Perim $=26.0^{\prime}$ r= $0.31{ }^{\prime}$ |
|  |  |  |  |  | $\mathrm{n}=0.030$ Earth, grassed \& winding |

14.5760 Total

## Subcatchment 2S: Basin 2



## Summary for Pond 1P: Pond



Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Peak Elev=290.04' @ 9.36 hrs Surf.Area= 5,254 sf Storage= 15,358 cf
Plug-Flow detention time $=139.5 \mathrm{~min}$ calculated for 1.572 af ( $100 \%$ of inflow)
Center-of-Mass det. time $=139.5 \mathrm{~min}(827.3-687.8)$


Discarded OutFlow Max=1.31 cfs @ 9.36 hrs HW=290.04' (Free Discharge)
$L_{1=E x f i l t r a t i o n ~(C o n t r o l s ~}^{1.31} \mathrm{cfs}$ )
Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=285.00' (Free Discharge)
$\iota_{2=B r o a d-C r e s t e d ~ R e c t a n g u l a r ~ W e i r ~(C o n t r o l s ~} 0.00 \mathrm{cfs}$ )

## Pond 1P: Pond



Pond 1P: Pond
Stage-Area-Storage


Time span=0.00-36.00 hrs, dt=0.01 hrs, 3601 points
Runoff by SBUH method, Split Pervious/Imperv.
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

## Subcatchment 1S: Basin 1

Subcatchment 2S: Basin 2

Pond 1P: Pond

## Summary for Subcatchment 1S: Basin 1

Runoff $=6.34$ cfs @ 8.00 hrs, Volume $=3.170$ af, Depth= 2.59"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-36.00 hrs, dt= 0.01 hrs Type IA 24-hr 100-Yr Rainfall=6.50"

|  | ea (sf) | CN | escription |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 62,500 | 98 R |  |  |  |
|  | 62,000 | 98 O | Roads \& Shared Driveway <br> On-Site Houses \& Driveways (34) |  |  |
|  | 5,500 | 49 P | Pervious - 50-75\% Grass cover, Fair, HSG A |  |  |
|  | 56,000 | 36 | Offsite - Woods, Fair, HSG A |  |  |
| 640,000441,500 |  | 61 | Weighted Average |  |  |
|  |  | 44 | 68.98\% Pervious Area |  |  |
| 198,500 |  | 983 | 31.02\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope $(\mathrm{ft} / \mathrm{ft})$ | Velocity (ft/sec) | Capacity $\qquad$ (cfs) | Description |
| 7.6 | 110 | 0.3500 | 0.24 | Sheet Flow, Sheet - Offsite Woods |  |
|  |  |  |  |  | Woods: Light underbrush n=0.400 P2=3.05" |
| 3.8 | 300 | 0.0700 | 1.32 |  | Shallow Concentrated Flow, Offsite Woods |
|  |  |  |  |  | Woodland Kv= 5.0 fps |
| 4.0 | 1,470 | 0.0720 | 6.06 | 48.46 | Channel Flow, Swale/Ditch |
|  |  |  |  |  | Area= 8.0 sf Perim=26.0' $\mathrm{r}=0.31{ }^{\prime}$ |
|  |  |  |  |  | $\mathrm{n}=0.030$ Earth, grassed \& winding |

15.4 1,880 Total

Subcatchment 1S: Basin 1


## Summary for Subcatchment 2S: Basin 2

Runoff $=\quad 2.99$ cfs @ 8.00 hrs, Volume= 1.281 af, Depth= 3.72"
Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-36.00 hrs, dt= 0.01 hrs Type IA 24-hr 100-Yr Rainfall=6.50"

|  | ea (sf) | CN | escription |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 15,500 | 98 | On-Site Roads |  |  |
|  | 44,000 | 98 | On-Site Houses \& Driveways (11) |  |  |
|  | 20,000 | 98 O | Off-Site Houses \& Driveways |  |  |
|  | 92,500 | 49 P | Pervious - 50-75\% Grass cover, Fair, HSG A |  |  |
| 8,000 |  | 98 | Off-Site - Future Highland Road |  |  |
| 180,000 |  | 73 | Weighted Average |  |  |
|  |  | 49 | 51.39\% Pervious Area |  |  |
| $87,500$ |  | 98 | 48.61\% Impervious Are |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | Capacity <br> (cfs) | Description |
| 13.3 | 280 | 0.2000 | 0.35 |  | Sheet Flow, Sheet - Lot Yards |
|  |  |  |  |  | Grass: Dense n=0.240 P2=3.05" |
| 1.2 | 480 | 0.0900 | 6.77 | 54.18 | Channel Flow, Future Road Ditch |
|  |  |  |  |  | Area= 8.0 sf Perim=26.0' $\mathrm{r}=0.31{ }^{\prime}$ |
|  |  |  |  |  | $\mathrm{n}=0.030$ Earth, grassed \& winding |

14.5760 Total

Subcatchment 2S: Basin 2


| Prepared by Blake Davis @ FDG | Printed $3 / 31 / 2021$ |
| :--- | ---: |
| HydroCAD® 10.00-24 $\mathrm{s} / \mathrm{n}$ M23544 © 2018 HydroCAD Software Solutions LLC | Page 10 |

## Summary for Pond 1P: Pond



Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Peak Elev= 294.28' @ 11.34 hrs Surf.Area= 10,040 sf Storage= 47,458 cf
Plug-Flow detention time= 255.4 min calculated for 4.449 af ( $100 \%$ of inflow)
Center-of-Mass det. time $=255.5 \mathrm{~min}$ (986.9-731.5)


Discarded OutFlow Max=2.62 cfs @ 11.34 hrs HW=294.28' (Free Discharge)
$L_{1=E x f i l t r a t i o n ~(C o n t r o l s ~}^{2.62}$ cfs)
Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=285.00' (Free Discharge)
${ }^{4}$ 2=Broad-Crested Rectangular Weir ( Controls 0.00 cfs )

## Pond 1P: Pond



Pond 1P: Pond
Stage-Area-Storage


## Channel Report

## Max Capacity - Ditch at 1.00\%

| Triangular |  |
| :--- | :--- |
| Side Slopes (z:1) | $=2.00,2.00$ |
| Total Depth (ft) | $=2.00$ |
|  | $=1.00$ |
| Invert Elev (ft) | $=1.00$ |
| Slope (\%) | $=0.080$ |
| N-Value |  |
|  |  |
| Calculations | Known Depth |
| Compute by: | $=2.00$ |

Highlighted

| Depth (ft) | $=2.00$ |
| :--- | :--- |
| Q (cfs) | $=13.79$ |
| Area (sqft) | $=8.00$ |
| Velocity (ft/s) | $=1.72$ |
| Wetted Perim (ft) | $=8.94$ |
| Crit Depth, Yc (ft) | $=1.25$ |
| Top Width (ft) | $=8.00$ |
| EGL (ft) | $=2.05$ |



| Depth | Q | Area | Veloc | Wp |
| :---: | :---: | :---: | :---: | :---: |
| (ft) | (cfs) | (sqft) | (ft/s) | (ft) |
| 0.10 | 0.005 | 0.020 | 0.23 | 0.45 |
| 0.20 | 0.030 | 0.080 | 0.37 | 0.89 |
| 0.30 | 0.088 | 0.180 | 0.49 | 1.34 |
| 0.40 | 0.189 | 0.320 | 0.59 | 1.79 |
| 0.50 | 0.342 | 0.500 | 0.68 | 2.24 |
| 0.60 | 0.556 | 0.720 | 0.77 | 2.68 |
| 0.70 | 0.839 | 0.980 | 0.86 | 3.13 |
| 0.80 | 1.198 | 1.280 | 0.94 | 3.58 |
| 0.90 | 1.640 | 1.620 | 1.01 | 4.02 |
| 1.00 | 2.172 | 2.000 | 1.09 | 4.47 |
| 1.10 | 2.801 | 2.420 | 1.16 | 4.92 |
| 1.20 | 3.532 | 2.880 | 1.23 | 5.37 |
| 1.30 | 4.373 | 3.380 | 1.29 | 5.81 |
| 1.40 | 5.328 | 3.920 | 1.36 | 6.26 |
| 1.50 | 6.405 | 4.500 | 1.42 | 6.71 |
| 1.60 | 7.607 | 5.120 | 1.49 | 7.16 |
| 1.70 | 8.943 | 5.780 | 1.55 | 7.60 |
| 1.80 | 10.42 | 6.480 | 1.61 | 8.05 |
| 1.90 | 12.03 | 7.220 | 1.67 | 8.50 |
| 2.00 | 13.79 | 8.000 | 1.72 | 8.94 |


| Yc | TopWidth | Energy |
| :---: | :---: | :---: |
| (ft) | (ft) | (ft) |
| 0.06 | 0.40 | 0.10 |
| 0.11 | 0.80 | 0.20 |
| 0.17 | 1.20 | 0.30 |
| 0.23 | 1.60 | 0.41 |
| 0.29 | 2.00 | 0.51 |
| 0.35 | 2.40 | 0.61 |
| 0.41 | 2.80 | 0.71 |
| 0.47 | 3.20 | 0.81 |
| 0.53 | 3.60 | 0.92 |
| 0.60 | 4.00 | 1.02 |
| 0.66 | 4.40 | 1.12 |
| 0.73 | 4.80 | 1.22 |
| 0.79 | 5.20 | 1.33 |
| 0.85 | 5.60 | 1.43 |
| 0.92 | 6.00 | 1.53 |
| 0.98 | 6.40 | 1.63 |
| 1.05 | 6.80 | 1.74 |
| 1.12 | 7.20 | 1.84 |
| 1.18 | 7.60 | 1.94 |
| 1.25 | 8.00 | 2.05 |

## Channel Report

## Max Capacity - 18in at 1.00\%

Circular
Diameter (ft) $\quad=1.50$

Invert Elev (ft)
Slope (\%)
N -Value
Calculations
Compute by:
Known Depth (ft) = 1.41

Highlighted

| Depth (ft) | $=1.41$ |
| :--- | :--- |
| Q (cfs) | $=13.35$ |
| Area (sqft) | $=1.72$ |
| Velocity (ft/s) | $=7.74$ |
| Wetted Perim (ft) | $=3.98$ |
| Crit Depth, Yc (ft) | $=1.37$ |
| Top Width (ft) | $=0.71$ |
| EGL (ft) | $=2.34$ |

Elev (ft)


## Max Capacity - 12in at 1.00\%

Circular
Diameter (ft) $\quad=1.00$

Invert Elev (ft)
Slope (\%)
N -Value
Calculations
Compute by:
Known Depth (ft)
$=1.00$
$=1.00$
$=0.011$

Known Depth
$=0.94$

Highlighted

| Depth (ft) | $=0.94$ |
| :--- | :--- |
| Q (cfs) | $=4.527$ |
| Area (sqft) | $=0.77$ |
| Velocity (ft/s) | $=5.91$ |
| Wetted Perim (ft) | $=2.65$ |
| Crit Depth, Yc (ft) | $=0.89$ |
| Top Width (ft) | $=0.47$ |
| EGL (ft) | $=1.48$ |

Elev (ft)

## Section



## Max Capacity - 12in at 2.00\%

Circular
Diameter (ft) $\quad=1.00$

Invert Elev (ft)
Slope (\%)
N -Value
Calculations
Compute by:
Known Depth (ft)
$=1.00$
$=2.00$
$=0.011$

Known Depth
$=0.94$

Highlighted

| Depth (ft) | $=0.94$ |
| :--- | :--- |
| Q (cfs) | $=6.402$ |
| Area (sqft) | $=0.77$ |
| Velocity (ft/s) | $=8.35$ |
| Wetted Perim (ft) | $=2.65$ |
| Crit Depth, Yc (ft) | $=0.97$ |
| Top Width (ft) | $=0.47$ |
| EGL (ft) | $=2.02$ |

Elev (ft)

## Section



## APPENDIX B

## Referenced Data

United States Department of Agriculture


Natural
Resources
Conservation
Service

A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

## Custom Soil Resource Report for

 Tillamook County, Oregon

## Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.
Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/ portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/? cid=nrcs142p2_053951).
Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.
Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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## Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.


## MAP LEGEND

| Area of Interest (AOI) |  |
| :--- | :--- |
| $\square$ | Area of Interest (AOI) |
| Soils |  |
| $\square$ | Soil Map Unit Polygons |
| $\square$ | Soil Map Unit Lines |
| $\square$ | Soil Map Unit Points |

Special Point Features
(c) Blowout

B Borrow Pit
次 Clay Spot
$\diamond$ Closed Depression
Bravel Pit
$\therefore \quad$ Gravelly Spot
(4) Landfill
A. Lava Flow
A. Marsh or swamp
\& Mine or Quarry
(-) Miscellaneous Water

- Perennial Water
- Rock Outcrop
+ Saline Spot
$\because \quad$ Sandy Spot
을 Severely Eroded Spot
- Sinkhole

3) Slide or Slip
(6) Sodic Spot

## MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.
Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Tillamook County, Oregon
Survey Area Data: Version 12, Jun 11, 2020

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: May 28, 2020—Jun 22, 2020

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background magery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

# Map Unit Legend 

| Map Unit Symbol |  | Map Unit Name | Acres in AOI |
| :--- | :--- | ---: | ---: |
| 6D | Horseprairie-Ferrelo complex, 3 <br> to 20 percent slopes | 15.3 | $21.0 \%$ |
| 11D | Netarts fine sandy loam, 5 to 30 <br> percent slopes | 23.5 | $32.4 \%$ |
| 11E | Netarts fine sandy loam, 30 to <br> 60 percent slopes | 25.5 | $35.2 \%$ |
| 20D | Klootchie-Necanicum complex, <br> 5 to 30 percent slopes | 1.8 | $2.4 \%$ |
| 20E | Klootchie-Necanicum complex, <br> 30 to 60 percent slopes | 5.1 | $\mathbf{7 . 0 \%}$ |
| 32D | Munsoncreek-Flowerpot <br> complex, 5 to 30 percent <br> slopes | 1.4 | $\mathbf{1 . 9 \%}$ |
| Totals for Area of Interest |  | $\mathbf{7 2 . 5}$ | $\mathbf{1 0 0 . 0 \%}$ |

## Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not
mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a soil series. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into soil phases. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.
A complex consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An association is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An undifferentiated group is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.
Some surveys include miscellaneous areas. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

## Tillamook County, Oregon

## 6D—Horseprairie-Ferrelo complex, 3 to 20 percent slopes

## Map Unit Setting

National map unit symbol: 280k
Elevation: 100 to 300 feet
Mean annual precipitation: 80 to 100 inches
Mean annual air temperature: 49 to 52 degrees $F$
Frost-free period: 180 to 300 days
Farmland classification: Farmland of statewide importance

## Map Unit Composition

Horseprairie and similar soils: 65 percent
Ferrelo and similar soils: 25 percent
Minor components: 5 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

## Description of Horseprairie

## Setting

Landform: Marine terraces
Landform position (three-dimensional): Riser
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Eolian and/or marine deposits

## Typical profile

Oi-0 to 2 inches: slightly decomposed plant material
A - 2 to 11 inches: medial loam
Bw1-11 to 28 inches: loam
Bw2 - 28 to 45 inches: loam
2C-45 to 62 inches: loamy sand
Properties and qualities
Slope: 3 to 20 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high
( 0.57 to $1.98 \mathrm{in} / \mathrm{hr}$ )
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: High (about 10.4 inches)

## Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 3e
Hydrologic Soil Group: B
Forage suitability group: Well Drained $<15 \%$ Slopes (G004AY014OR)
Other vegetative classification: Well Drained <15\% Slopes (G004AY014OR),
Sitka spruce/oxalis, swordfern-moist (902)
Hydric soil rating: No

## Description of Ferrelo

## Setting

Landform: Marine terraces
Landform position (three-dimensional): Riser
Down-slope shape: Convex
Across-slope shape: Linear, convex
Parent material: Eolian and/or marine deposits

## Typical profile

Oi-0 to 1 inches: slightly decomposed plant material
A - 1 to 19 inches: loam
Bw-19 to 37 inches: loam
2C1-37 to 55 inches: loamy fine sand
2C2-55 to 89 inches: fine sand
Properties and qualities
Slope: 3 to 20 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high
( 0.57 to $1.98 \mathrm{in} / \mathrm{hr}$ )
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Moderate (about 8.7 inches)
Interpretive groups
Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 3e
Hydrologic Soil Group: B
Forage suitability group: Well Drained $<15 \%$ Slopes (G004AY014OR)
Other vegetative classification: Well Drained <15\% Slopes (G004AY014OR),
Sitka spruce/oxalis, swordfern-moist (902)
Hydric soil rating: No

## Minor Components

Depoe
Percent of map unit: 5 percent
Landform: Depressions on marine terraces
Hydric soil rating: Yes

## 11D—Netarts fine sandy loam, 5 to 30 percent slopes

## Map Unit Setting

National map unit symbol: 27w3
Elevation: 20 to 300 feet
Mean annual precipitation: 80 to 100 inches

Mean annual air temperature: 49 to 52 degrees $F$
Frost-free period: 180 to 300 days
Farmland classification: Farmland of statewide importance

## Map Unit Composition

Netarts and similar soils: 90 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

## Description of Netarts

## Setting

Landform: Dunes on marine terraces
Landform position (three-dimensional): Tread
Down-slope shape: Linear, concave
Across-slope shape: Linear
Parent material: Eolian sands
Typical profile
Oi-0 to 2 inches: slightly decomposed plant material
A-2 to 5 inches: fine sandy loam
$E-5$ to 9 inches: loamy fine sand
ABs - 9 to 15 inches: loamy fine sand
Bs1-15 to 19 inches: fine sand
Bs2-19 to 37 inches: fine sand
BCs - 37 to 54 inches: fine sand
$C$ - 54 to 67 inches: fine sand
Properties and qualities
Slope: 5 to 30 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Capacity of the most limiting layer to transmit water (Ksat): High (1.98 to 5.95 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Low (about 4.4 inches)
Interpretive groups
Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 4e
Hydrologic Soil Group: A
Other vegetative classification: Sitka spruce/salal-mesic (901)
Hydric soil rating: No

## 11E—Netarts fine sandy loam, 30 to 60 percent slopes

## Map Unit Setting

National map unit symbol: 280q
Elevation: 20 to 300 feet
Mean annual precipitation: 80 to 100 inches
Mean annual air temperature: 49 to 52 degrees $F$

Frost-free period: 180 to 300 days
Farmland classification: Not prime farmland

## Map Unit Composition

Netarts and similar soils: 90 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

## Description of Netarts

Setting
Landform: Dunes on marine terraces
Landform position (three-dimensional): Tread
Down-slope shape: Linear, concave
Across-slope shape: Linear
Parent material: Eolian sands

## Typical profile

Oi-0 to 2 inches: slightly decomposed plant material
A-2 to 5 inches: fine sandy loam
$E-5$ to 9 inches: loamy fine sand
$A B s-9$ to 15 inches: loamy fine sand
Bs1-15 to 19 inches: fine sand
Bs2-19 to 37 inches: fine sand
BCs - 37 to 54 inches: fine sand
C - 54 to 67 inches: fine sand

## Properties and qualities

Slope: 30 to 60 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Capacity of the most limiting layer to transmit water (Ksat): High (1.98 to 5.95 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Low (about 4.4 inches)
Interpretive groups
Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 6e
Hydrologic Soil Group: A
Other vegetative classification: Sitka spruce/salal-mesic (901)
Hydric soil rating: No

## 20D—Klootchie-Necanicum complex, 5 to $\mathbf{3 0}$ percent slopes

## Map Unit Setting

National map unit symbol: 27xq
Elevation: 50 to 1,800 feet
Mean annual precipitation: 80 to 110 inches
Mean annual air temperature: 46 to 52 degrees F
Frost-free period: 120 to 210 days

Farmland classification: Not prime farmland

## Map Unit Composition

Klootchie and similar soils: 60 percent
Necanicum and similar soils: 25 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

## Description of Klootchie

## Setting

Landform: Mountain slopes
Landform position (two-dimensional): Summit, toeslope
Landform position (three-dimensional): Mountaintop, mountainbase
Down-slope shape: Concave
Across-slope shape: Concave, linear
Parent material: Colluvium and residuum derived from igneous rock and tuff
Typical profile
Oi -0 to 1 inches: slightly decomposed plant material
A1-1 to 9 inches: medial silt loam
A2-9 to 19 inches: medial silt loam
Bw1-19 to 44 inches: medial silty clay loam
Bw2-44 to 68 inches: medial silty clay loam

## Properties and qualities

Slope: 5 to 30 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high
( 0.57 to $1.98 \mathrm{in} / \mathrm{hr}$ )
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Very high (about 19.1 inches)
Interpretive groups
Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 6 e
Hydrologic Soil Group: B
Other vegetative classification: Sitka spruce/salmonberry-wet (903)
Hydric soil rating: No

## Description of Necanicum

## Setting

Landform: Mountain slopes
Landform position (two-dimensional): Summit, footslope
Landform position (three-dimensional): Mountaintop, mountainbase
Down-slope shape: Convex, linear
Across-slope shape: Convex, linear
Parent material: Colluvium derived from igneous rock and tuff

## Typical profile

Oi-0 to 1 inches: slightly decomposed plant material
A1-1 to 10 inches: very gravelly medial loam
A2-10 to 18 inches: very gravelly medial loam
Bw1-18 to 27 inches: very gravelly medial loam

Bw2 - 27 to 49 inches: extremely cobbly medial loam
Bw3-49 to 71 inches: extremely cobbly medial loam

## Properties and qualities

Slope: 5 to 30 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high
( 0.57 to $1.98 \mathrm{in} / \mathrm{hr}$ )
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Moderate (about 7.8 inches)

## Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 6 e
Hydrologic Soil Group: B
Other vegetative classification: Sitka spruce/salmonberry-wet (903)
Hydric soil rating: No

## 20E—Klootchie-Necanicum complex, 30 to $\mathbf{6 0}$ percent slopes

## Map Unit Setting

National map unit symbol: 27x3
Elevation: 50 to 1,800 feet
Mean annual precipitation: 80 to 110 inches
Mean annual air temperature: 46 to 52 degrees $F$
Frost-free period: 120 to 210 days
Farmland classification: Not prime farmland

## Map Unit Composition

Klootchie and similar soils: 55 percent
Necanicum and similar soils: 30 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

## Description of Klootchie

## Setting

Landform: Mountain slopes
Landform position (two-dimensional): Backslope, footslope
Landform position (three-dimensional): Center third of mountainflank, lower third of mountainflank
Down-slope shape: Concave
Across-slope shape: Concave, linear
Parent material: Colluvium and residuum derived from igneous rock and tuff

## Typical profile

Oi-0 to 1 inches: slightly decomposed plant material
A1-1 to 9 inches: medial silt loam
A2-9 to 19 inches: medial silt loam

Bw1 - 19 to 44 inches: medial silty clay loam
Bw2 - 44 to 68 inches: medial silty clay loam

## Properties and qualities

Slope: 30 to 60 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high
( 0.57 to $1.98 \mathrm{in} / \mathrm{hr}$ )
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Very high (about 19.1 inches)

## Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 6 e
Hydrologic Soil Group: B
Other vegetative classification: Sitka spruce/oxalis, swordfern-moist (902)
Hydric soil rating: No

## Description of Necanicum

## Setting

Landform: Mountain slopes
Landform position (two-dimensional): Backslope, footslope
Landform position (three-dimensional): Upper third of mountainflank, lower third of mountainflank
Down-slope shape: Linear, convex
Across-slope shape: Convex, linear
Parent material: Colluvium derived from igneous rock and tuff

## Typical profile

Oi-0 to 1 inches: slightly decomposed plant material
A1-1 to 10 inches: very gravelly medial loam
A2-10 to 18 inches: very gravelly medial loam
Bw1-18 to 27 inches: very gravelly medial loam
Bw2-27 to 49 inches: extremely cobbly medial loam
Bw3-49 to 71 inches: extremely cobbly medial loam

## Properties and qualities

Slope: 30 to 60 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high ( 0.57 to $1.98 \mathrm{in} / \mathrm{hr}$ )
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Moderate (about 7.8 inches)

## Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 6 e
Hydrologic Soil Group: B
Other vegetative classification: Sitka spruce/oxalis, swordfern-moist (902)
Hydric soil rating: No

## 32D—Munsoncreek-Flowerpot complex, 5 to 30 percent slopes

## Map Unit Setting

National map unit symbol: 27zw
Elevation: 50 to 1,800 feet
Mean annual precipitation: 80 to 110 inches
Mean annual air temperature: 46 to 52 degrees F
Frost-free period: 120 to 210 days
Farmland classification: Not prime farmland

## Map Unit Composition

Munsoncreek and similar soils: 65 percent
Flowerpot and similar soils: 20 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

## Description of Munsoncreek

## Setting

Landform: Hillslopes, mountain slopes
Landform position (two-dimensional): Footslope, summit
Landform position (three-dimensional): Mountainbase, mountaintop, base slope, interfluve
Down-slope shape: Linear, convex
Across-slope shape: Convex, linear
Parent material: Colluvium and residuum derived from sedimentary rock

## Typical profile

Oi-0 to 1 inches: slightly decomposed plant material
A-1 to 10 inches: medial silt loam
$A B-10$ to 18 inches: silty clay loam
Bw1-18 to 28 inches: silty clay loam
Bw2-28 to 41 inches: silty clay loam
Bw3-41 to 58 inches: extremely paragravelly silty clay loam
$\mathrm{Cr}-58$ to 68 inches: weathered bedrock

## Properties and qualities

Slope: 5 to 30 percent
Depth to restrictive feature: 40 to 60 inches to paralithic bedrock
Drainage class: Well drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20 to $0.57 \mathrm{in} / \mathrm{hr}$ )
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: High (about 11.4 inches)

## Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 6e
Hydrologic Soil Group: C

Other vegetative classification: Sitka spruce/salmonberry-wet (903)
Hydric soil rating: No

## Description of Flowerpot

## Setting

Landform: Hillslopes, mountain slopes
Landform position (two-dimensional): Toeslope, summit
Landform position (three-dimensional): Mountainbase, mountaintop, interfluve, base slope
Down-slope shape: Concave
Across-slope shape: Concave, linear
Parent material: Colluvium and residuum derived from sedimentary rock

## Typical profile

Oi-0 to 1 inches: slightly decomposed plant material
A1-1 to 8 inches: medial silty clay loam
A2-8 to 14 inches: silty clay loam
$A B-14$ to 22 inches: silty clay loam
Bw - 22 to 30 inches: silty clay loam
Bg - 30 to 52 inches: silty clay loam
$B C-52$ to 60 inches: silty clay loam

## Properties and qualities

Slope: 5 to 30 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Somewhat poorly drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high ( 0.06 to $0.57 \mathrm{in} / \mathrm{hr}$ )
Depth to water table: About 14 to 22 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Very high (about 13.1 inches)

## Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 6e
Hydrologic Soil Group: C/D
Other vegetative classification: Sitka spruce/salmonberry-wet (903)
Hydric soil rating: No

## APPENDIX A - HYDRAULIC ROUGHNESS (MANNING'S n) VALUES OF CONDUITS AND CHANNELS

This appendix lists Manning's roughness (n) values for various conduits and channels, as follows:

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TABLE 1: CONDUITS

| Conduit | HYDRAULIC ROUGHNESS (MANNING'S n) VALUES |  |  |
| :---: | :---: | :---: | :---: |
|  | Minimum | Normal | Maximum |
| A. Concrete or asbestos-cement pipe | 0.011 | 0.013 | 0.015 |
| B. Metal pipe or pipe-arch with annular corrugations |  |  |  |
| 1. $2-2 / 3$-inch $\mathrm{x} \frac{1 / 2-i n c h ~ c o r r u g a t i o n s ~}{\text { a }}$ |  |  |  |
| a. Plain or fully coated |  | 0.024 |  |
| b. Paved invert (range represents 25 and 50 percent of circumference paved, with larger $n$ value representing 25 percent paved) |  |  |  |
| 1. Full flow depth | 0.018 | .... | 0.021 |
| 2. Flow 80 percent of depth | 0.016 | . . . . | 0.021 |
| 3. Flow 60 percent of depth | 0.013 | .... | 0.019 |
| 2. 3-inch $x 1$-inch corrugations | $\ldots$. | 0.027 | $\cdots$ |
| 3. 6-inch x 2-inch corrugations | $\ldots$... | 0.032 | $\ldots$. |
| C. Smooth walled helical spiral rib pipe | 0.012 |  | 0.013 |
| D. Corrugated metal subdrain | 0.017 | 0.019 | 0.021 |
| E. Plastic pipe USE $n=0.011$ FOR NEW CWVVEPIS |  |  |  |
|  | $0.011$ |  | 0.015 |
| 2. Corrugated |  | 0.024 | ..... |
| F. Metal pipe or pipe arch with helically wound corrugations |  |  |  |
| 1. Smaller pipes |  |  |  |
| 12 inch |  | 0.013 | .... |
| 15 inch |  | 0.014 | . . . ${ }^{\text {. }}$ |
| 18 inch |  | 0.015 | . . . ${ }^{\text {a }}$ |

TABLE 5: EXCAVATED ARTIFICIAL CHANNELS

| Channel | HYDRAULIC ROUGHNESS (MANNING'S n) VALUES |  |  |
| :---: | :---: | :---: | :---: |
|  | Minimum | Normal | Maximum |
| A. Earth, straight and uniform |  |  |  |
| 1. Clean, recently completed | 0.016 | 0.018 | 0.020 |
| 2. Clean, after weathering | 0.018 | 0.022 | 0.025 |
| 3. Gravel, uniform section, clean | 0.022 | 0.025 | 0.030 |
| 4. With short grass, few weeds | 0.022 | 0.027 | 0.033 |
| B. Earth, winding and sluggish |  |  |  |
| 1. No vegetation | 0.023 | 0.025 | 0.030 |
| 2. Grass, some weeds | 0.025 | 0.030 | 0.033 |
| 3. Dense weeds or aquatic plants in deep channels | 0.030 | 0.035 | 0.040 |
| 4. Earth bottom and rubble sides | 0.028 | 0.030 | 0.035 |
| 5. Stony bottom and weedy banks | 0.025 | 0.035 | 0.040 |
| 6. Cobble bottom and clean sides | 0.030 | 0.040 | 0.050 |
| C. Dragline-excavated or dredged |  |  |  |
| 1. No vegetation | 0.025 | 0.028 | 0.033 |
| 2. Light brush on banks | 0.035 | 0.050 | 0.060 |
| D. Rock cuts |  |  |  |
| 1. Smooth and uniform | 0.025 | 0.035 | 0.040 |
| 2. Jagged and irregular | 0.035 | 0.040 | 0.050 |
| E. Channels not maintained, weeds and brush uncut | ROADSIDE DTTCH- |  | Acoumen |
| 1. Dense weeds, high as flow depth | 0.050 | 0.080 | 0.120 |
| 2. Clean bottom, brush on sides | 0.040 | 0.050 | 0.080 |

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February $12^{\text {th }}, 2021$
Report \# 21-0008

Bill Hughes<br>Avalon Heights LLC<br>41901 Old Highway 30<br>Astoria, OR 97103

REGARDING: Stormwater Infiltration Test, Avalon Heights, Netarts-Oceanside, Oregon T: 1S, R: 10W, SW ¼ SE ¼ Section 30, TL 200

Dear Mr. Hughes,
As requested, Environmental Management Systems Inc. (EMS) has performed the following services and provides this report for your use.

## PROJECT DESCRIPTION:

The purpose of this report is to document the results of soil infiltration testing and to determine the potential for onsite stormwater disposal. The subject property is a $21.20-$ acre lot located near Oceanside, Oregon. A 56-lot subdivision is planned for the property and must be developed in accordance with Tillamook County Development Standards. On February $5^{\text {th }}, 2020$, EMS conducted a soil infiltration test in the proposed stormwater infiltration area near the south end of the property. This report describes existing site conditions, methods used, and results.

## SUMMARY:

Onsite stormwater infiltration appears feasible. The average infiltration rate was 21.45 inches per house. No cementation or restrictive layers were observed in the test pit which was dug to a depth of 30". The stormwater infiltration facility should be engineered in manner that prevents erosion and does not cause instability of the steep slopes on the site.

## LIMITATIONS:

Findings and recommendations in this report are based infiltration testing performed in one location. Conditions encountered during the test are believed to be representative of the site conditions, however subsurface conditions may vary across the site. If there
are changes to the plan that involve infiltrating stormwater elsewhere onsite, additional testing may be required.

## SITE CONDITIONS:

## Existing Uses for the Property

The site is currently undeveloped but was logged within the last couple of years.

## Topography

The site is an irregularly shaped lot that sits on top of a large hill (stable dune) at elevations ranging between 300 and 430 feet above sea level. The terrain is rolling hills with an overall southward facing slope. Slopes are variable across the site with the majority western part of the site being less than $20 \%$. A broad gulley-like depression runs through the center of the property from north to south that serves as a seasonal drainageway or infiltration swale (see photos 1 and 2, below). The southern end of this gulley is topographically the lowest area on the site and is proposed to be used for infiltrating stormwater runoff for the subdivision.


Photo 1 The gulley-like drainageway, facing north.


Photo 2 The drainageway, facing south. The infiltration test hole is shown in the lower center of frame.

## Site Stability

The site is mapped as a moderate to high landslide hazard area by Oregon Department of Geology and Mineral Industries (DOGAMI) ${ }^{1}$. According to DOGAMI Statewide Landslide Information Layer for Oregon (SLIDO) ${ }^{2}$ there is a large area of landslide topography near the east property line that extends to the east. No instability or landslide activity was observed during the site visit. See Geological Hazard Report prepared by EMS on April $12^{\text {th }}, 2018$ for more details.

## Vegetation

Most of the site has been logged, but previously the vegetation on site consisted of a mix of conifers including Douglas-fir (Pseudotsuga menziesii), Sitka spruce (Picea sitchensis), and western hemlock (Tsuga heterophylla). The drainage swale is still

[^0]vegetated predominantly with red alder (Alnus rubra), sword fern (Polystichum munitum), salal (Gaultheria shallon), and huckleberry (Vaccinium spp.).

## Soils

Soils on site are mapped as 11D and 11E - Netarts fine sandy loam (5-30 percent slopes and 30-60 percent slopes respectively) by the Natural Resource Conservation Service (NRCS) ${ }^{3}$. The typical setting for this soil type is dunes on marine terraces with a parent material of eolin sands. This unit is described as well drained with the depth to restrictive feature being more than 80 inches. According to NRCS, the typical soil profile is as follows:

0 to 2 inches: slightly decomposed plant material
2 to 5 inches: fine sandy loam
5 to 15 inches: loamy fine sand
15 to 67 inches: fine sand
For the soil infiltration test, one $24^{\prime \prime}$ by $36^{\prime \prime}$ test pit was dug to a depth of 30 " and the soil profile was evaluated prior to conducted the test. One inch of slightly decomposed plant material was observed at the soil surface. 1 inch to 30 inches from the soil surface is somewhat silty fine sand. No cementation or restrictive layers were observed. Medium roots were common and extended to the bottom of the pit.

## Wetlands / Surface Water

No surface water was observed during the site assessment. No wetlands are mapped on the site by the National Wetland Inventory (US Fish \& Wildlife). There is no local wetland inventory available for the Oceanside-Netarts area. Obligate wetland vegetation was not observed in the stormwater infiltration area.

## METHODS:

One $24^{\prime \prime}$ by $36^{\prime \prime}$ test pit was dug to a depth of $30^{\prime \prime}$ near the bottom of the proposed infiltration facility. Water for the infiltration test was provided by Netarts-Oceanside Fire district. Precipitation data was obtained from a nearby weather station (TILLAMOOK 6.9 SSE, OR). The month of January had received 19.02 inches of precipitation which is approximately $140 \%$ of normal ${ }^{1}$ for that month. The vicinity had received approximately 3.5 inches of rain over the 4 days prior to the test. Therefore, the pit was not presoaked.

An open-pit falling-head test was conducted twice. The falling head test was prepared by filling the pit to a known depth ( $15^{\prime \prime}$ from the bottom) and measuring the time it took to recede to the bottom of the pit using a stopwatch. Between tests \#1 and \#2, the constant head flow rate was measured using a 5-gallon bucket and stopwatch and determined to be 5.43 gallons per minute.

[^1]
## RESULTS:

Results of the infiltration tests are shown in Table 1, below. The average infiltration rate is 21.45 inches per hour.
Table 1. Infiltration Test Results

| Test | Measured Infiltration Rate | Inches per Hour |
| :--- | :--- | :--- |
| $\# 1$ | $15^{\prime \prime} / 39$ minutes | 23.4 |
| $\# 2$ | $15^{\prime \prime} / 46.3$ minutes | 19.5 |

## CONCLUSION:

Infiltration in the area of the drainage swale is fairly rapid, therefore onsite infiltration of stormwater for the proposed subdivision appears feasible. The stormwater facility will need to be sized appropriately to manage stormwater for all new impervious surfaces created by the project and will need to be constructed in a manner that will not cause erosion and instability at the bottom of the slope.

DISCLOSURE: The information and statements in this report are true and accurate to the best of our knowledge. Neither Environmental Management Systems, Inc., nor the undersigned have any economic interests in the project.

Thank you for your business, and we look forward to assisting you to achieve your development objectives. If you have any questions, please contact me at 503-3539691.


Emma Eichhorn, REHS
Environmental Health Specialist
ENVIRONMENTAL MANAGEMENT SYSTEMS, Inc.

## Enclosed:

Site plan


## APPENDIX C

Basin Map


## ANALON HEGhtTS SUBDIVISION

December 8, 2005

## PROCESS TO DEVELOP DESIGN STANDARDS Using AASHTO Manual

Reference Documents:
"A POLICY on GEOMETRIC DESIGN of HIGHWAYS and STREETS - 2001" by American Association of State Highway and Transportation Officials (AASHTO Manual). As the AASHTO Manual is updated the most current edition applies as appropriate.
"TILLAMOOK FIRE DISTRICT UNIFORM FIRE CODE", Appendix III-E (Fire Department Access). This document was updated by the Fire Defense Board on 4/21/05. Some section numbering and standards were updated accordingly.

This handout is meant to provide clarification of procedures used to determine road standards when applying the AASHTO Manual (see below). This handout is not designed to replace actual use and reference of the AASHTO Manual. Note that if other County Ordinances or the local fire departments have higher standards than AASHTO, those other standards apply.

The purpose of this analysis is to develop the design features for a "Local" rural road functional classification. This type of road is consistent with "Major Local" and "Minor Local" roads. For design features for "Minimum Local" roads (i.e. a street accessing 4 or less residences), see Page 3 of this handout.

DEFINITIONS FOR TERRAIN (Page 235 of AASHTO Manual)
"Level" terrain is where highway sight distances, as governed by both horizontal and vertical restrictions, are generally long or could be made to be so without construction difficulty or major expense.
"Rolling" terrain is where the natural slopes consistently rise above and fall below the road grade and where occasional steep slopes offer some restriction to normal horizontal and vertical roadway alignment.
"Mountainous" terrain is where longitudinal and transverse changes in the elevation of the ground with respect to the road are abrupt and where benching and side hill excavations are frequently required to obtain acceptable horizontal and vertical alignment.

## DEVELOPMENT OF DESIGN STANDARDS for "MAJOR LOCAL" AND "MINOR LOCAL" ROADS

1. Determine the design Average Daily Traffic (ADT).
a. For most developments this will be the existing ADT plus additional ADT for the developments being considered which are causing the needed improvements.
b. When considering land development as a cause of increased ADT, the increased ADT should assume the maximum density allowed by the zoning.
c. For residential developments the ADT is assumed to be 10 vehicles per day per residence.
2. Determine the Design Speed.
a. Use Table 5-1 (Page 385).
b. Use Terrain definitions listed above.
3. Determine Stopping Sight Distance and "K" Values for Vertical Curves: Use Table 5-2 (Page 385).
4. No sections with passing sight distances are required with typical subdivision or major partition roads.
5. Determine Maximum Grade: Use Table 5-4 (Page 386).
6. Roadway Cross Slope. (Use $2 \%$ crown unless otherwise needed for curve superelevation or alternate engineered design)
7. Determine Superelevation and Maximum Degree of Curve.
a. Maximum superelevation is $12 \%$ (Page 387).
b. Use Tables 3-21 through 3-25 (Pages 157-165) horizontal curvature design. If an applicant proposes a curve radius inconsistent with these tables, they need to provide engineered design details subject to approval by the Director of Public Works.
8. Determine Minimum Width of Traveled Way (paved road): Use Table 5-5 (Page 388).
9. Determine Width of Graded Shoulder (each side).
a. Use Table 5-5 (Page 388).
b. In Mountainous Terrain shoulder in roadway cuts may be deceased by 2 feet, but only if:
(1) the total roadway width is not less than 18 feet, and
(2) the cut is not on the inside of a minimum radius curve, and
(3) stopping sight distance is not impaired by the roadway cut.


Exhibit 5-1. Minimum Design Speeds for Local Rural Roads

a Rate of vertical curvature, K , is the length of curve per percent algebraic difference in the intersecting grades (ie., $K=L / A$ ). (See Chapter 3 for details.)

Exhibit 5-2. Design Controls for Stopping Sight Distance and for Crest and Sag Vertical Curves

## ROARING TIE LOOP BETWEEN highland a ocean song <br> = ALL OTHER PROPOSED

Rate of vertical curvature, K , is the length of curve per percent algebraic difference in the intersecting grades (i.e., $K=L / A$ ). (See Chapter 3 for details.)

Exhibit 5-3. Design Controls for Crest Vertical Curves Based on Passing Sight Distance

## Grades

Suggested maximum grades for local rural roads are shown in Exhibit 5-4.

| Type of terrain | Metric |  |  |  |  |  |  |  |  | US Customary |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Maximum grade (\%) for specified design speed (km/h) |  |  |  |  |  |  |  |  | Maximum grade (\%) for specified design speed (mph) |  |  |  |  |  |  |  |  |
|  | 20 | 30 | 40 | 50 | 60 | 70 | 80 | 90 | 100 | 15 | 20 | 25 | 30 | 40 | 45 | 50 |  | 60 |
| Level | 9 | 8 | 7 | 7 | 7 | 7 | 6 | 6 | 5 |  | 8 | 7 | 7 | 7 | 7 | 6 |  | 5 |
| Rolling | 12 | 11 | 11 | 10 | 10 | 9 | 8 | 7 | 6 |  | 11 | 11 | 10 | 10 | 9 | 8 |  | 6 |
| Mountainous | 17 | 16 |  | 14 |  | 12 | 10 | 10 | - | 17 | 16 | 15 | 14 | 13 |  |  |  |  |

Exhibit 5-4. Maximum Grades for Local Rural Roads

## Alignment

Alignment between control points should be designed to be as favorable as possible consistent with the environmental impact, topography, terrain, design traffic volume, and the amount of reasonably obtainable right-of-way. Sudden changes between curves of widely different radii or between long tangents and sharp curves should be avoided. Where practical, the design should include passing opportunities. Where crest vertical curves and horizontal curves occur together, there should be greater than minimum sight distance to ensure that the horizontal curves are visible to approaching drivers.


Exhibit 5-5. Minimum Width of Traveled Way and Shoulders

## ROARNG TDE LOOP BETWEEN Highland HOD OCEAN SONS: 18 ' traveled way +2 ' Shoulders <br> ALL OTHER PROPOSED PADS: <br> $18^{\prime}$ traversed way + 2'Shouldeps

From: Melissa Jenck
Sent: Wednesday, April 10, 2019 2:08 PM
To: DEBLASI Michael; 'mark@meadeng.com'; 'bchexc@gmail.com'
Cc: Sarah Absher
Subject: RE: Avalon Heights 2nd Addition

Michael,
Thank you for the update!

## Melissa Jenck

From: DEBLASI Michael [michael.deblasi@state.or.us](mailto:michael.deblasi@state.or.us)
Sent: Wednesday, April 10, 2019 1:44 PM
To: 'mark@meadeng.com' [mark@meadeng.com](mailto:mark@meadeng.com); 'bchexc@gmail.com' [bchexc@gmail.com](mailto:bchexc@gmail.com); Melissa Jenck [mjenck@co.tillamook.or.us](mailto:mjenck@co.tillamook.or.us)
Subject: Avalon Heights 2nd Addition
Melissa,
I visited the site on April $5^{\text {th }}$ to determine if there were any jurisdictional Waters of the State. During my inspection of the site, I did not determine any portion of the waterway or adjacent lands to be jurisdictional to the Department of State Lands. The waterway is ephemeral and no wetlands were observed where the project impacts are proposed to occur.

Michanel De Planii<br><br>

Oregon Department of State Lands
775 Summer St NE, Suite 100
Salem, Ore 97303
503.986 .5226
http://mww.oregon.gov/DSL/Pages/index.aspx
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wa: 360-735-1109
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4080 SE International Way
Suite B112
Milwaukie, OR 97222
April $12^{\text {th }}, 2018$
EMS Report \#18-0005

Bill Hughes<br>Avalon Heights LLC<br>41901 Old Highway 30<br>Astoria, OR 97103

REGARDING: Geohazard Report, Proposed Avalon Heights Subdivision, Oceanside, Tillamook County, Oregon, T: 1S, R: 10W, Sec: 30, TL:200.

## Project Description

Environmental Management Systems, Inc. (EMS) has prepared this Geological Hazard Report for the proposed residential subdivision to be developed on the 21.20-acre tax lot listed above. This report has been prepared to satisfy Tillamook County Land Use Ordinance Article IV, Section 4.130. EMS reviewed aerial photographs, Oregon Department of Geology and Mineral Industries (DOGAMI) LIDAR imagery, geologic hazard maps, landslide inventory data, US Geological Survey (USGS) geologic maps, Natural Resource Conservation Service (NRCS) soil maps, and a previous geohazard report prepared by EMS in 2005. On February $3^{\text {rd }}, 2018$ EMS conducted an onsite geologic hazard reconnaissance to visually evaluate surface conditions at the project site. The attached Figures 1 through 10 are maps of the site and vicinity including: 1) Tax Lot Map, 2) 2017 Aerial Map, 3) USGS Topographic Map, 4) USGS Geology Map, 5) NRCS Soil Map, 6) LIDAR Map, 7) SLIDO Landslide Map, 8) Coastal Dune Hazard Map, 9) Existing Conditions and 10) Conceptual Site Plan. Selected photographs showing site features are also attached. Conditions during the site reconnaissance were cloudy with light precipitation and temperatures around 55 degrees Fahrenheit.

The subject property, referred to in this report as the "site" is a 21.20-acre lot located near the unincorporated town of Oceanside, Oregon. It is identified as Tax Lot 200 in Section 30DC, Township 1 South and Range 10 West in Tillamook County (see Figure 1 - Tax Lot Map).

The site is currently undeveloped, but a 67-lot subdivision is proposed. See Figure 10 Conceptual Site Plan. The subject property is zoned as ROS - Residential Oceanside Zone, which permits single-family dwellings, public parks, and on-site manufactured homes. Lot sizes are dictated by Tillamook County Zoning ordinance and depend on existing grades. Lots with average slopes of less than 19\% must be at least 7,500 square feet; lots with slopes 20-29\% must be at least 10,000 square feet; lots with slopes greater than $29 \%$ must be at least 20,000 square feet. Public sanitary sewer, water and electric utilities are available.

## Landscape Setting and Land Use

As mentioned above, the site is undeveloped but has been used for timber cultivation and harvest as recently as 2007. The site is situated less than a mile southeast of Oceanside's town
center and about 1-mile northwest of Netarts. Neighboring lots along the site's western property line are all developed with single family residences, but areas to the north, east, and west are mostly undeveloped forestland. There is one large single-family residence to the east that is accessed by a private road that runs through the lot. The Netarts-Oceanside Sanitary District wastewater treatment plant is approximately 0.25 miles northeast of the site. Directly south of the southwest portion of the parcel are remnant foundation supports for a large above ground water storage tank, on top of which is a new smaller tank. The Oceanside Water District water tower is directly north of the site near Highland Drive and is accessed via an easement through the site. Both tanks can be seen in the Figure 2 - Aerial Map. A Netarts Water District water line runs across the northern part of the site (see Image 1).

The site is in Land Resource Region 4A-Sitka Spruce Belt which is typically dominated by Sitka spruce, western hemlock, western red cedar, Douglas-fir, salal, huckleberry, and swordfern ${ }^{1}$. The average annual precipitation is $52-60$ inches which is evenly distributed throughout the fall, winter and spring, in contrast to a relatively dry summer. The average annual temperature is $45-55$ degrees Fahrenheit. This site is covered by Scotch Broom, heavy brush, and stands of mixed evergreen trees. The site was logged sometime around 2007 so trees are $<24^{\prime \prime}$ diameter.

The elevation for the subject parcel varies between 300 and 430 feet above sea level with higher elevations in the north part of the parcel. Most of the site is made up of rolling, gentle to moderate slopes aside from the far eastern part of the lot which is much steeper ranging between approximately 40 and 60 percent and descending down to perennial stream. According to the USGS Topo map for the Netarts Quadrangle, this stream appears to be a tributary of Fall Creek which runs along the east property line from northeast to southwest and drains into the Pacific Ocean. An unnamed, intermittent drainageway runs through the center of the lot from north to south. Topographic features including nearby waterways can be seen in Figure 3 Topographic Map. Figure 9 - Site Plan depicts the topographic survey of the property from 2005.

## Analysis

Bedrock and soils
The gentle to moderately sloped terrain that extends about a mile inland from the coast is primarily underlain by unconsolidated Quaternary-age beach, dune, marine terrace and river deposits of variable thickness of up to over 100 feet. These deposits in the site region are mapped by Schlicker and others (1972) ${ }^{2}$ as being a stable sand dune formation. Later mapping of the region by Wells and others (1995) ${ }^{3}$ identifies the area as beach and dune deposits (map unit Qb; Figure 4 - USGS Geology Map). This unit is described as unconsolidated moderately well sorted, fine to medium grained beach sand and well sorted, cross bedded fine grained sand comprising active and inactive dune ridges; locally includes basalt gravel and boulder deposits derived from rocky headlands and fine fluvial and lacustrine mud behind coastal dune ridges.

The Quaternary deposits overly Miocene age sedimentary and volcanic bedrock in the site region. Bedrock units include the Grande Ronde Basalt flows mapped north and east from the site as well as in isolated cliff-forming areas along the coast west from the site (map unit Tgr; Figure 40 USGS Geology Map). Miocene-age sedimentary rock units include the Astoria

[^2]Formation (map unit Tac) to the southeast and Sandstone of Cape Mears (map unit Tcm) to the northeast. Review of the geologic map by Wells and others (1995) suggest bedrock beneath the Quaternary deposits at the site is more likely to be basalt flows, however marine sedimentary rocks may be present (see Figure 4). The dune sand may also overly old marine terrace deposits that have been uplifted and formed on the eroded bedrock surface. The thickness of the dune deposits and depth to bedrock is not known at this time. Subsurface explorations are recommended to evaluate soil and bedrock conditions across the site.

The Wells and others (1995) geologic map shows bedrock faults are present in the region and provides some altitudes of bedding in the sedimentary rocks and joints or interflow zones in the basalt lava flows. Bedding in the Astoria Formation is generally dipping westerly between 10 and 25 degrees and generally northerly dips of between 8 and 15 degrees in the Sandstone of Cape Mears and the basalt flows (Figure 4)

According to USDA Natural Resource Conservation Service, soils on the subject property are Netarts fine sandy loam ${ }^{4}-5-30$ percent slopes in the western half of the lot, and 30-60 percent slopes in the eastern half of the lot (see map units 11D and 11E in Figure 6 - NRCS Soils Map). The landform for Netarts fine sandy loam is dunes on marine terraces. It is not rated as hydric and is in hydrological soil Group A. NRCS describes Netarts fine sand as being well drained with depth to restrictive feature or water table being more than 80 inches from the surface.

The typical soil profile for the Netarts fine sandy loam is as follows:
Oi -0 to 2 inches: slightly decomposed plant material
A - 2 to 5 inches: fine sandy loam
E-5 to 9 inches: loamy fine sand
ABs - 9 to 15 inches: loamy fine sand
Bs1-15 to 19 inches: fine sand
Bs2-19 to 37 inches: fine sand
BCs -37 to 54 inches: fine sand
C-54 to 67 inches: fine sand
During a site reconnaissance in November 2005, a 3-inch diameter, 60-inch-long AMS soil auger was used to excavate and expose the soils to maximum depth of the soil auger. Four excavations were completed within the boundary limits of the parcels. No restrictive layer or water table was reached down to 60 inches (maximum depth of the auger). Borings 1 and 2 were found to be fine sand down to $56^{\prime \prime}$ which weak to moderate structure and slight to moderate cementation. 16-40" was slightly restrictive to groundwater percolation with a nonrestrictive layer beneath. Borings 3 and 4 were silt loam from 0 " to 30 " and 0 " to $32^{\prime \prime}$, respectively, followed by silty clay to $60^{\prime \prime}$. The structure was weak, friable, and slightly plastic in the silt loam layer and moderate to strong blocky and not restrictive in the silty clay layer. Data from that investigation including boring locations are attached at the end of this report.

No subsurface explorations were made during this recent site assessment, but surface conditions appeared unchanged since 2005. 1 to 2 feet of soil (fine sand) was exposed at a road cut near the east side of the property (see Image 2). Discontinuous iron-cemented sand also observed in some places (see Image 3). Iron cemented layers may be laterally extensive where seasonal or perennial perched water tables may form. Perched groundwater may also be present where silty clay soils are present. As noted later in this report we recommend

[^3]Page 3 of 15
subsurface explorations and testing to provide geotechnical data to further evaluate grading plans (cuts and fills), compaction, storm water systems, and slope stability, for example.

## Slope and water drainage patterns

Figure 9 shows existing topographic contours. Slopes generally descend from north to south and include broad north to south oriented ridges and swales. The majority upper, western part of the site is fairly flat but with rolling hill topography (see Images 4 and 5). Approximately 100 feet from the east property line slopes increase to 45-60\% (see Image 4). According to the USGS Topographic Map for the area, a perennial stream is at the bottom of this steep slope which extends off site.

Based on LIDAR imagery (Figure 6) and on-site observations of site topography, water appears to flow from northeast to southwest toward the Pacific Ocean and Netarts Bay. The USGS Topo 7.5-minute map for Netarts, Oregon indicates the presence of an unnamed stream or creek runs that through the center of the subject property from north to south. A broad north-south oriented valley-like depression or seasonal swale was observed there during the site assessment but no water present. Another unnamed creek is mapped at the foot of the slope just east of the property. This appears to be a tributary of the north branch of Fall Creek which drains into the Pacific Ocean to the southwest. No standing or flowing water was observed anywhere on site during the site assessment. The National Flood Insurance program maps this area "undetermined by possible flood hazard"5. The site does not appear to be prone to flooding.

Landslide inventory and identification of visible landslide activity in the immediate area According to DOGAMI Statewide Landslide Information Layer for Oregon (SLIDO ${ }^{6}$ ) and mapping by Schlicker and others ${ }^{7}$ (1972), a large area of landslide topography is present at the eastern edge of the property (See Figure 7 - Landslide Map). No landslide topography or obvious indications of slope instability or landslide activity was observed during the site reconnaissance. Conifer trees were dominantly straight throughout the site and the ground did not appear to be slumping. No scarps from recent landslide activity were observed. No cracks were observed in the foundation of neighboring properties from Highland Drive.

## History of dune erosion

Based on review of the Oregon HazVu: Statewide Geohazards Viewer (DOGAMI, https://gis.dogami.oregon.gov/hazvu/), this site is not located in a coastal erosion hazard area. No evidence of dune erosion was identified on the site, but coastal erosion is rated between low and very high closer to the ocean west of the site ${ }^{8}$. Figure 8 - Coastal Erosion Hazard Map shows the risk of coastal erosion for the area.

During the winter of 1997-1998, an active beach margin was eroded at the base of the dune complex where the nearby Capes Subdivision is located west of the site. According to Wes Greenwood's 2005 geologic hazard report ${ }^{9}$, the erosion allowed storm generated waves to

[^4]erode a foredune that was located at the base of the large, upslope dune complex. A large, slow moving landslide mass approximately 500 feet wide and 900 feet long resulted and imperiled homes constructed on the upper, west portion of the dune complex, located directly upslope from the landslide scarp. Engineering measures such as soil pins have been placed along the scarp to stabilize the existing dwellings. However, the toe of the landslide has not been protected and future storms may further erode the beach margin and activate another landslide.

## Recommended development standards

## Development density

Development density should be consistent with Tillamook County's Zoning Ordinance for Residential Oceanside Zone.

## Locations for structures and roads

Setback of home structures from roads, property lines, and other structures should abide by Tillamook County's Zoning Ordinance for the ROS zone (Section 3.310(4)(d-g)). Figure 10 Conceptual Site Plan shows the proposed limits of the 67 lots and street locations. Buildings should be set back at least 50 feet from the steep slope break in the eastern part of the site. Development on slopes greater than $50 \%$ should be avoided.

## Land grading practices, including standards for cuts and fills

Grading on the upper, western part of the lot is proposed to reduce slopes if necessary for development. A topographic survey should be done to verify current slope conditions across the site prior to the development of a grading plan. Figure 10 - Conceptual Site Plan shows proposed topographic contours and is based on the prior survey. Engineered retaining wall(s) along the eastern steep slope or elsewhere may be necessary for development. Specifications for grading including cuts and fills, soil compaction and drainage, will be provided in a geotechnical report following recommended subsurface explorations across the site.

## Vegetation removal and re-vegetation practices

Vegetation should be maintained as much as possible where construction is proposed. Clearing of the easternmost part of the lot where the steepest slopes are located should be avoided and reserved as an open space.

## Foundation design

Home structures can be placed on typical spread footings where excavated subgrade should consist of medium stiff to hard soil or weathered bedrock and including removal of organic topsoil and undocumented fill if present. Significantly deeper footings or foundation supported on piles may be necessary in steeper slope (> 29\%) areas.

Road design
Road design should be consistent with Tillamook County Development Standards.
Management of storm water run-off during and after construction
Runoff and erosion should be controlled during and after construction to prevent erosion or create unstable soil conditions at the site. During construction, silt fences should be placed around the construction area, and wattles should be placed at the base of slopes to treat runoff. Exposed soil should be covered with straw during construction and immediately replanted afterward to prevent sheet and rill erosion. Storm water management recommendations for runoff from roof, driveways, and other impervious surface on each lot and the new streets will be evaluated following additional geotechnical investigations that are recommended. The conceptual storm water plan shown on Figure 10 assumes that stormwater conveyance pipes
will be installed under new roads and directed to a stormwater attenuation pond in the southern portion of the site.

## Geotechnical report

Geotechnical investigations are recommended to provide additional information needed for final design. The results will be included in a future geotechnical report for submittal to Tillamook County. The investigations will include a detailed topographic map of the site that will be used to refine the grading plan. Subsurface explorations and testing should be completed to characterize soil, bedrock and groundwater conditions across the site. This information is needed to further evaluate seismic design criteria, slope stability, grading plans, specifications for cuts and fills, retaining wall design, groundwater and storm water management plans.

## Summary findings and conclusions

The following addresses each of the summary findings and conclusions required by TCLUO Section 4.130(8):
a. Type of proposed use and adverse effects it might have on the surrounding areas
b. Hazards to life, public and private property, and the natural environment that may be caused by the proposed use
c. Methods for protecting the surrounding area from adverse effects of the development
d. Temporary and permanent stabilization programs and the planned maintenance of new and existing vegetation
e. The proposed development is adequately protected from any reasonably foreseeable hazards including but not limited to geologic hazards, wind erosion, undercutting and flooding
f. The proposed development is designed to minimize adverse environmental effects

The subject property is located on an undeveloped "stable dune" hillside dominated by a mix of young conifers and dense understory brush. The site is about 0.5 miles east of the Pacific Ocean and is not within the coastal erosion hazard zone or tsunami inundation zone. Evidence that the site is unstable was not observed during the site assessment, although development on the steepest slopes will require proper design and construction to maintain slope stability.

Both a topographic survey and professional land survey should be conducted prior to construction to accurately delineate setbacks and steep slope breaks. Geotechnical investigations and a geotechnical report are recommended to provide information needed to further evaluate grading plans, lot and street layout, slope stability, storm water management, and site specific standards for design and construction. The future scope of work will include review of subsurface data and reports from the Capes Subdivision, nearby water tanks, as well as geotechnical hole, monitoring well, and water well reports available from the Oregon Department of Water Resources.

It is my opinion that a 67-lot subdivision can safely be developed. Additional recommendations and site specific standards will be provided in the geotechnical report that incorporates the recommendations summarized above and will further address relevant geologic hazards, storm water and vegetation management.

## Limitations

The opinions and recommendations contained in this report are not intended as a warranty but are offered to assist you in the planning and design process. The report is based on field observations and background review only. Subsurface explorations, soils testing, and geologic
and engineering analysis may be necessary to confirm our interpretation of subsurface conditions and more fully develop the required level of detail and engineering for design and construction of a home structure.

DISCLOSURE: The information and statements in this report are true and accurate to the best of our knowledge. Neither Environmental Management Systems, Inc., nor the undersigned have any economic interests in the project.

Thank you for your business, and we look forward to assisting you to achieve your development objectives.

Sincerely,


John Jenkins, CEG
Oregon Certified Engineering Geologist No. E1119

Enclosed:<br>2005 Boring Log Data<br>Ground Level Color Photographs<br>Figure 1 - Tax Lot Map<br>Figure 2-2017 Aerial Map<br>Figure 3 - USGS Topographic Map<br>Figure 4 - USGS Geology Map<br>Figure 5 - NRCS Soils Map<br>Figure 6 - Lidar Map<br>Figure 7 - Landslide Map<br>Figure 8 - Coast Dune Map<br>Figure 9 - Existing Conditions<br>Figure 10 - Conceptual Site Plan



Measured slopes and soil boring locations (1-4) from the 2005 geologic hazard assessment

# ENVIRONMENTAL MANAGEMENT SYSTEMS, INC.-SOILS BORING DATA 

 (Bay City, OR Offree) 7304 Biseline Rpad, Bay City, OR 97107 Pbonc: (503)-812-9655 FAK: (503) 377 -0324

APPLICANI: My'hre Group Architects
PREVIOUS EVALUATION: () Yes ( X ) Ne
$\because$ ADDRESS OF SITE: Dindevefoped Parcel Ocen

SUBDIVISION: $\qquad$
WORKLD. 在: 05:5072

LEGAL DESCRIPTION: TOWNSFIP: $1 S$
RANGE: HOW
SECTION: 3ODC. TAXLOT: 200 ;
Date: 9-Noveraber 2605 16.Noverulber 2005

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| Evaluated 9 Navesalber 2005 | $1.6{ }^{\prime \prime}-40^{\prime \prime}$ | ES | 5YR 4/6 |  |  |  | Kiany fine | Moderaté, granular, noderate cementanion; Slightresmiction to groundwater percolation |
|  | $40^{\prime \prime}-56^{\prime \prime}$ | ES | .JOYR 714 |  | . |  | Few fine | Weak, gramular, slight cementation; Nol Restrictive to groundivater percolation |
| Slope: 12\% |  |  |  |  |  |  |  |  |
| \#2 | 0-16 | fS | 10YR 7/3 |  |  |  | Many firie | Weak, granular, small laminir lenses of $\mathrm{FeO}_{2}$ (5YR.470); slight cementation; Not Restrictive |
| EvalnatedO November 2005 | $16^{\prime \prime}-38^{\prime \prime}$ | 2S | 5YR 4/6 |  | . |  | Many' fine | Moderate, gramularir moderate cementation; Slight restiiction to groundwater percolation |
|  | $38^{\prime \prime}-56^{\prime \prime}$ | 2S | 10YR 7/4. |  |  |  | Few fine | Weak, granular, slight cementation; Not Restrictive to groumdwater percolation |
|  |  |  |  |  |  |  |  |  |
|  | $0-30^{\circ}$ | S] | 10YR4/3 |  |  |  | Many coarse \& fine | Weak, firabie; Sliginly plastic; Common organic inclusions at surface; fine $S$ throughout solat imatrix |
| Evaluated 16 November 2005 | $30^{\prime \prime}-60^{\prime \prime}$ | SiCl | 10YR6/4 |  |  | . | Many <br> $\rightarrow$ Fev! <br> coiarse <br> \& Ine | Moderate to Strong Blocigy; Slight to moderate plasticity; Strong clay films; Not restrictive to proundwater percolationi |
|  |  |  |  |  |  |  |  |  |
| 44 | $0-32^{* *}$ | SI | 10YR4/3 |  | . |  | Many coarse \& fine | Weak, friable; Slighly plastice:Common organic inclusions at surface; fine $S$. throughont soli matrix |
| Evaluated 16 Novensber 2005 | $32^{\prime \prime}$ - 60" | SiCl | 10YR6/4 | . | $\cdots$ | - | $\begin{gathered} \text { Many } \\ \rightarrow \text { Few } \\ \text { coarse } \\ \text { \& fine } \end{gathered}$ | Moderate to Strong Blocky; Slight to moderate plasticity; Strong clay films; Not restrictive to groundwater percolation |
| Slope: 18\% | ESD $\geq 60^{\prime \prime}$ NO Temp. Percliig H2O $\rightarrow 60$ |  |  |  |  |  |  |  |

ENYIRONMIENTAL MANAGEMENT SYSTEMS, INC. -SOLLS BORING DATA
 (Bay City, OR Officel 7304 Baseline Road, Bay Cilv, OR 97107 Phone: (503) 872-9655 FAX: (5033) 377-0324



Image 1 Sewer and water are available for the site. A Netarts water district water line runs through the northern part of the site.


Image 2: 1-2 feet of soil was exposed at a roadcut approximately in the middle north of the site. Soil on the site is deep, fine sand with slight to moderate cementation.


Image 3: Iron cemented sand was observed in some places. This photo was taken facing west near the top of the steep slope in the eastern part of the site.


Image 4: Most of site is fairly flat (<19\%) but with variable slopes and rolling hill topography.


Image5: Most of site is fairly flat (<19\%) but with variable slopes and rolling hill topography.

## Page $\mathbf{1 4}$ of $\mathbf{1 5}$



Image 6: A view of the site facing east. This photo was taken from the top of the steep slope in the eastern part of the site.
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R ASSESSMENT AND TAXAL
LEGAL, ENGINEERIN

Tillamook Courty
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SEE MAP IS 10W 30


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CHECKED:

DRAWN:

| AVALON HEIGHTS LLC | SCALE: 1 " $=200$ |
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| T: 1S, R:10W, S: 30 TL:200 | 10 APR 2018 |
| NRCS SOILS MAP | PAGE 5 OF 10 |



|  | Cheeces: | AVALON HEIGHTS LLC | SCALE: $1^{\prime \prime}=300$ |
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|  | DRNM: | T: 1S, R:10W, S: 30 TL:200 | 10 APR 2018 |
| ENVIRONMENTAL |  | LIDAR MAP | PAGE 6 OF 10 |



| 5 |
| :---: | :---: | :---: | :---: |



4080 SE International Way Suite B112
Milwaukie OR 97222

ENVIRONMENTAL MANAGEMENT SYSTEMS, INC

CHECKED:

DRAW:



## CHECKED:

## DRAWN:

| AVALON HEIGHTS LLC |
| :---: |
| T: 1S, R:10W, S: 30 TL:200 |
| CONCEPTUAL SITE PLAN |

SCALE: $1^{\prime \prime}=200^{\prime}$
10 APR 2018

## Tillamook County



# DEPARTMENT OF COMMUNITY DEVELOPMENT 

 BUILDING, PLANNING \& ON-SITE SANITATION SECTIONS
## DATE:



TO: Tillamook County One-Stop Permit Counter
FROM:
NETARTS WATER DISTRICT
RE: SW前Water Availability (Circle)
Water Availability Valid For Six Months
Dear Sir:

I confirm that
Township: $1 S$ Range 100 Section 30 DC Tax Lot 00200
According to our records, the legal owner is: AVALON HEIGHTS LLC


This letter shall not create a liability on the part of Tillamook County, or by an officer, or employee thereof, for the-services described above.
there, for there.

r
inner Dusuncas Nanserer Signature and Title of Authorized Representative
(503) 842-9405
(503) $842-9380$

FAX
cc: Property Owner

G:Planning FormsiSewer-Water.Ltr

Preliminary Plat Application Narrative Supplement for<br>Second Addition to Avalon Heights

The initial application narrative either explained how the application met the preliminary plat submission requirements by describing how the various facets or design features were consistent with the criteria and/or standards, or referred the reader to the appropriate drawing sheet in the application drawing set where an illustration and labeled dimension would be more effective. This supplement addresses the preliminary plat approval criteria of land division ordinance Section 070.

## SECTION 070: PRELIMINARY PLAT APPROVAL CRITERIA

(1) Approval Criteria. The Approval Authority (Director for partitions and Planning Commission for subdivisions) may approve, approve with conditions or deny a preliminary plat. The Approval Authority decision shall be based on findings of compliance with all of the following approval criteria:
(a)The land division application shall conform to the requirements of this ordinance;

Applicant Response: The initial narrative demonstrated that all required items of Section 060 are included on the preliminary plat drawings or otherwise attached to the application package. These include lot sizes, block lengths, utility sizes and locations, fire hydrants and street lights, road widths, required setbacks and conceptual building foot prints, etc. We believe these items make the application conform to the ordinance requirements.
(b)All proposed lots, blocks, and proposed land uses shall conform to the applicable provisions of the Land Use Ordinance - Article 3 Zone Regulations and the standards in Section 150 of this ordinance;

Applicant response: The proposed development is to accommodate the construction of single family dwellings and the subject property is zoned Residential Oceanside where such structures are permitted outright. Lot sizes vary from near the minimum area size to over 20,000 square feet as required my Tillamook County Land Use Ordinance 3.310 to accommodate the site slopes.
(c)Access to individual lots, and public improvements necessary to serve the development, including but not limited to water, sewer and streets, shall conform to the standards in Sections 150 and 160 of this ordinance;

Applicant Response: All proposed lots will be accessed by private roads or shared driveways to be constructed by the applicant. All utilities will be located within the right-of-way/easement
with stubs to every lot. A separate series of easements will provide for storm water conveyance and infiltration. Please see Sheet 4.
(d)The proposed plat name is not already recorded for another subdivision, does not bear a name similar to or pronounced the same as the name of any other subdivision within the County, unless the land platted is contiguous to and platted by the same party that platted the subdivision bearing that name or unless the party files and records the consent of the party that platted the contiguous subdivision bearing that name;

Applicant Response: Because of the original Avalon Heights subdivision is abuts the subject property to the west, this proposal is named "Second Addition to Avalon Heights" which has been_suggested previously by the county surveyor.
(e)The proposed streets, utilities, and surface water drainage facilities conform to Tillamook County's adopted master plans and applicable engineering standards and, within Unincorporated Community Boundaries, allow for transitions to existing and potential future development on adjacent lands. The preliminary plat shall identify all proposed public improvements and dedications;

Applicant Response: Review of the preliminary plat drawing set all roads are designed to county standards, but will remain privately maintained, as will the storm water management system. Water and sewer facilities will be dedicated to the Netarts Water District and the Netarts-Oceanside Sanitary District, respectively. See attached letter and certificate. An access easement will be granted to provide access to Tax Lots 100 and 101 to the east. All other adjacent property is zoned Forestry.
(f)All proposed private common areas and improvements, if any, are identified on the preliminary plat and maintenance of such areas is assured through appropriate legal instrument;

Applicant Response: A Home Owners Association or similar organization will be formed and its responsibilities and authorities documented and recorded to provide a mechanism and financial resources to maintain the private storm water system and roads.
(g)Provisions for access to and maintenance of off-right-of-way drainage, if any;

Applicant Response: Access to the conveyance swale and infiltration facility is provided by $W$ Grand Avenue and Roaring Tide Loop. See Sheet 4.
(h)Evidence that any required State and Federal permits, as applicable, have been obtained or can reasonably be obtained prior to development; and

Second Addition to Avalon Heights
Preliminary Plat Application Narrative Supplement
Page 2

Applicant Response: There are no portions of the site that would require federal or state permits. Please see the letter from the Department of State Lands attached to this document.
i)Evidence that improvements or conditions required by the road authority, Tillamook County, special districts, utilities, and/or other service providers, as applicable to the project, have been or can be met, including but not limited to: (i) Water Department/Utility District Letter which states that the partition or subdivision is either entirely excluded from the district or is included within the district for purposes of receiving services and subjecting the partition or subdivision to the fees and other charges of the district

Applicant Response: The attached letter and certificate from the Netarts-Oceanside Sanitary District and the Netarts Water District demonstrate that water and sewer facilities can be installed and serve the proposed development. The power company was contacted by the applicant and they indicated that they would prefer to receive the application directly from the County, review the proposal, and make its determination at that time.
(ii) Subsurface sewage permit(s) or site evaluation approval(s) from the appropriate agency.

Applicant Response: This requirement does not apply because the development will be served by public sewer.

# lancaster mobley 



# Second Avalon Heights Subdivision 

> Transportation Impact Study
Oceanside, Oregon
Date:
April 1, 2021
Prepared for:
Bill Hughes
Prepared by:
Melissa Webb, PE
Todd Mobley, PE

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## Executive Summary

1. The proposed Second Avalon Heights subdivision is located east of Highland Drive W in Oceanside, Oregon. The development will include the construction of 60 single-family housing units on currently undeveloped land.
2. The trip generation calculations show that the proposed development is projected to generate up to 59 additional site trips during the evening peak hour and up to 618 additional site trips on a typical weekday.
3. Based on the most recent five years of crash data, no significant trends or crash patterns were identified at the study intersection that are indicative of safety concerns. Accordingly, no safety mitigation is recommended per the crash data analysis.
4. Adequate sight distance is available, or can be made available (with proper maintenance or removal of roadside vegetation along Highland Drive W), to ensure safe operation for northbound and southbound approaching vehicles at the site access intersection.
5. Due to insufficient traffic volumes, preliminary traffic signal warrants are not projected to be met at the unsignalized intersection of OR-131 at Highland Drive W under any of the analysis scenarios. In addition, leftturn lane warrants and right-turn lane warrants are not projected to be met at the study intersection under any of the analysis scenarios.
6. All study intersections are currently operating acceptably per ODOT standards and are projected to continue operating acceptably through the 2023 buildout year, regardless of the potential increase in site trip generation upon development of the site. No operational mitigation is necessary or recommended at these intersections.

## Project Description

## Introduction

The proposed Second Avalon Heights will include the subdivision of a property on tax lot 200 in Oceanside, Oregon. The project will include the construction of 60 single-family housing units on currently undeveloped land. This report addresses the impacts of the proposed subdivision on the nearby street system. Based on correspondence with Tillamook County and ODOT staff, the report conducts safety and capacity/level of service analyses at the following intersections:

1. OR-131 at Highland Drive W

The purpose of this study is to provide an analysis of potential traffic impacts of the proposed Second Avalon Heights subdivision on the surrounding transportation system and to recommend any required mitigative measures. Detailed information on traffic counts, trip generation calculations, safety analyses, and level of service calculations are included in the appendix to this report.

## Location Description

The project site is located northeast of the intersection of OR-131 at Highland Drive W in Oceanside, Oregon. The existing tax lot (tax lot 200) is currently undeveloped. The proposed $\pm 21$-acre development will include the construction of 60 single-family housing units. The development will take vehicular access via a proposed access point along Highland Drive W as well as an extension of Highland Drive W. The project site is shown in Figure 1.


Figure 1: Project Location (image from Google Earth)

## Vicinity Roadways

The proposed development is expected to impact two roadways near the site. Table 1 provides a description of each of the vicinity roadways.

Table 1: Vicinity Roadway Descriptions

| Street <br> Name | Functional <br> Classification | Cross- <br> Section | Speed <br> (MPH) |  <br> Sidewalks | On-Street <br> Parking | Bicycle <br> Facilities |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| OR-131 | District Highway | 2 -3 Lanes | 35 Posted | None | Not <br> Permitted | None |
| Highland <br> Drive W | Local Road | 2 Lanes | 20 <br> Statutory* | None | Permitted <br> Both Sides | None |

Table Notes: Functional Classification provided by the Oregon Transportation Map ${ }^{1}$ for Tillamook County and Oregon Highway Plan *Highland Drive W is an existing unpaved gravel road through a residential area. Applicant plans to pave the roadway as part of the proposed development.

## Study Intersections

Based on the location of the subject property, preliminary calculations of trip generation, and coordination with Tillamook County and ODOT, the intersection of OR-131 at Highland Drive W was identified for analysis. A summarized description of the study intersection is provided in Table 2.

Table 2: Study Intersection Descriptions

| Number | Intersection | Geometry | Traffic Control | Phasing/Stopped <br> Approaches |
| :---: | :---: | :---: | :---: | :---: |
| 1 | OR-131 at Highland Drive W | Three- <br> Legged | Stop-Controlled | WB Stop- <br> Controlled |

A vicinity map showing the project site, vicinity streets, and study intersection configurations is shown in Figure 2.

[^5]
## LEGEND

STUDY INTERSECTION- STOP SIGN
$\triangle$ PROJECT SITE
- COLLECTOR ROADWAY


## - LOCAL ROADWAY


(Future site access upon development)


Figure 2

## Site Trips

## Trip Generation

The proposed Second Avalon Heights will include the construction of a residential subdivision, consisting of 60 single-family housing units. The site is currently undeveloped.

To estimate the number of trips that will be generated by the proposed development, trip equations from the Trip Generation Manual ${ }^{2}$ were used. Data for land use code 210, Single-Family Detached Housing, was used to estimate the proposed development's trip generation based on the number of dwelling units.

Based on demographic information provided by the Tillamook County Planning Department, as well as correspondence with ODOT and Tillamook County staff, a "rental rate" reduction in trip generation was applied. According to the 2018 Oceanside Community Plan³, approximately $10 \%$ of residential lots are licensed as vacation rental units and are typically not occupied year-round. When rental units are empty, there would be no vehicle trips applied to the transportation system. While rental units are likely to be fully booked and occupied on weekends, it was assumed that rentals would be only half-booked on weekdays. As a result, a $5 \%$ reduction in trip generation volumes was applied to adjust for rental units which are not occupied during the weekday, and thus not contributing vehicle trips to the transportation system.

The trip generation calculations show that the proposed development is projected to generate up to 59 additional site trips during the evening peak hour and up to 618 additional site trips on a typical weekday. The trip generation calculations are summarized in Table 3 and detailed calculation worksheets are provided in the appendix.

Table 3: Trip Generation Summary

| Land Use | ITE Code | Size | Evening Peak Hour |  |  | Weekday |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | In | Out | Total | Total |
| Single-Family Detached Housing | 210 | 60 units | 39 | 23 | 62 | 650 |
| Rental Rate Reduction (5\%) |  |  | 2 | 1 | 3 | 32 |
| Net Increase |  |  | 37 | 22 | 59 | 618 |

## Trip Distribution

The directional distribution of site trips to and from the proposed site was estimated based on the locations of likely trip origins and destinations, was well as locations of major transportation facilities in the site vicinity. The following trip distribution was estimated and used for analysis:

- Approximately 50 percent of site trips will travel to/from the north along OR-131;
- Approximately 50 percent of site trips will travel to/from the south along OR-131;

[^6]While some site trips coming to and from the north could potentially use Grand Avenue to access the proposed subdivision, Grand Avenue has deteriorated due to small creeks of water carving their way throughout the road. Highland Drive W, while an unpaved gravel roadway, is shorter and offers a smoother ride. In addition, the applicant plans to pave Highland Drive W as part of the proposed development. Based on these observed roadway characteristics, it was assumed that all site trips would use Highland Drive W as the main roadway to access the Second Avalon Heights subdivision.

The trip distribution and assignment for the net site trips generated during the evening peak hour are shown in Figure 3.
LEGEND
$\left\langle\begin{array}{|c|c|c|c|}\hline X X \%\end{array}\right.$ PERCENT OF PROJECT TRIPS

| NET TRIP GENERATION |  |  |  |
| :---: | :---: | :---: | :---: |
|  | IN | OUT |  |
| PM | 37 | 22 |  |



Figure 3

## Traffic Volumes

## Existing Conditions

Traffic counts were conducted at the study intersection on Tuesday, June 9, 2020, from 3:00 PM to 6:00 PM. The intersection of OR-131 at Highland Drive W is a three-legged intersection; however, Capes Drive is located across and offset from Highland Drive W. Traffic associated with Capes Drive was included as part of the traffic count due to the proximity of the intersection. Turning movements and volumes were separated between the two intersections to determine the existing volumes at the study intersection.

The traffic counts at the study intersection were collected after the COVID-19 viral pandemic had become a public health concern throughout the state of Oregon. As a result, there has been a noticeable decline in traffic volumes on the transportation system that are atypical of normal conditions. In order to reflect normal traffic conditions without the impacts of the COVID-19 viral pandemic, traffic counts were adjusted.

With guidance from Tillamook County and ODOT staff, two methods were used to develop the 2021 existing $30^{\text {th }}$ highest hour turn movement volumes. The turn movement volumes from each method were compared and the highest turn movement volumes were used for a conservative analysis. The following methodology was used:

- Method 1: Counts taken on Saturday, July 15, 2006, were adjusted to bring the counts to 2021 existing $30^{\text {th }}$ highest hour turn movement volumes. At the request of ODOT staff, a linear growth rate of one percent per year was applied to the through movements along OR-131 over a 15-year period to determine year 2021 existing volumes. For all other turning movements, a linear growth rate of one-half percent per year was applied to the 2006 traffic volumes over a 15-year period to determine year 2021 existing volumes (refer to the Background Conditions section regarding the methodology used for determining traffic growth). In addition, ODOT staff requested that a seasonal adjustment factor (SAF) not be applied to the 2006 counts.
- Method 2:
- Existing Counts: As described above, traffic counts were collected at the study intersection of OR131 at Highland Drive W on Tuesday, June 9, 2020. A linear growth rate of one percent per year was applied to the through movements along OR-131 over a one-year period to determine year 2021 existing volumes. For all other turning movements, a linear growth rate of one-half percent per year was applied to the traffic volumes over a one-year period to determine year 2021 existing volumes
- COVID-19 Adjustment Factor: A COVID-19 adjustment factor was calculated by comparing 2019 and 2020 traffic counts collected at the Rockaway ATR 29-001 and Port Orford ATR 08-009. Based on the average difference in volumes, an adjustment factor of 1.45 was applied to all turning movements to bring the existing June 2020 counts to pre-COVID conditions.
- Seasonal Adjustment Factor: Since OR-131 is under the jurisdiction of ODOT, procedures described in ODOT's Analysis Procedures Manual ${ }^{4}$ (APM) were used to seasonally adjust existing traffic

[^7]volumes to reflect the $30^{\text {th }}$-highest hour in a typical year. Using a map of seasonal trends, this portion of OR-131 was determined to show a Coastal Destination Route trend. A seasonal adjustment factor (SAF) of 1.2576 was subsequently calculated and applied to the June 2020 COVID-adjusted through volumes along OR-131.

After comparing adjusted counts using the two methods outlined above, Method 1 produced the highest turn movement volumes. Therefore, these turning movements were used for a conservative analysis.

The existing adjusted evening peak hour traffic volumes at the study intersection is shown in Figure 4.

## Background Conditions

To provide analysis of the impact of the proposed development, an estimate of future traffic volumes is required. A growth rate must be applied to COVID-adjusted traffic volumes in order to calculate year 2023 background volumes.

Growth rates for through traffic on OR-131 were derived using ODOT's 2038 Future Volume Table. Data corresponding to Milepost 2.14 (ODOT Highway 131) was used for the intersection of OR-131 at Highland Drive W.

A growth factor of 1.0059 was applied to OR-131 through volumes over a two-year period to determine year 2023 background volumes.

For non-ODOT facilities, a growth rate of one-half percent per year was applied to the existing traffic volumes over a two-year period to determine year 2023 background volumes.

Figure 5 shows the projected year 2023 background traffic volumes at the study intersections during the evening peak hour.

## Buildout Conditions

Peak hour trips calculated to be generated by the proposed development, as described earlier within the Site Trips section, were added to the projected year 2023 background traffic volumes to obtain the expected year 2023 site buildout volumes.

Figure 6 shows year 2023 buildout traffic volumes at the study intersections during the evening peak hour.


## TRAFFIC VOLUMES



TRAFFIC VOLUMES

lancaster mobley

## TRAFFIC VOLUMES

Year 2023 Buildout Conditions
PM Peak Hour

Figure 6

## Safety Analysis

## Crash History Review

Using data obtained from ODOT's Crash Data System, a review of approximately five years of the most recent available crash history (January 2014 through December 2018) was performed at the study intersection. The crash data was evaluated based on the number of crashes, the type of collisions, and the severity of the collisions.

The intersection of OR-131 at Highland Drive W had no reported crashes during the analysis period, whereby no significant trends or crash patterns were identified at the study intersection that are indicative of safety concerns. Accordingly, no safety mitigation is recommended per the crash data analysis.

## Warrant Analysis

Left-turn lane warrants, right-turn lane warrants, and preliminary traffic signal warrants were examined for the intersection of OR-131 at Highland Drive W.

## Left-Turn Lane Warrants

A left-turn refuge lane is primarily a safety consideration for the major street, removing left-turning vehicles from the through traffic stream. The left-turn lane warrants were examined using methodologies provided in the ODOT's Analysis Procedures Manual (APM). Left-turn lane warrants were evaluated based on the number of advancing and opposing vehicles, number of turning vehicles, travel speed, and the number of through lanes.

Due to insufficient traffic volumes, left-turn lane warrants are not projected to be met at the intersection of OR131 at Highland Drive W under any of the analysis scenarios.

## Right-Turn Lane Warrants

Due to insufficient traffic volumes, right-turn lane warrants are not projected to be met at the intersection of OR-131 at Highland Drive W under any of the analysis scenarios.

## Preliminary Traffic Signal Warrants

Preliminary traffic signal warrants were examined for the unsignalized intersection of OR-131 at Highland Drive W to determine whether the installation of a new traffic signal will be warranted at the intersection upon completion of the proposed development:

Due to insufficient traffic volumes, traffic signal warrants are not projected to be met at the intersection of OR131 at Highland Drive W under any of the analysis scenarios.

## Sight Distance Evaluation

Intersection sight distance was examined for the proposed site access intersection of Highland Drive W at Roaring Tide Loop. Sight distance was measured and evaluated in accordance with standards established in $A$ Policy of Geometric Design of Highways and Streets. ${ }^{5}$ According to AASHTO, the driver's eye is assumed to be 15

[^8]feet from the near edge of the nearest travel lane of the intersecting street and at a height of 3.5 feet above the minor-street approach pavement. The vehicle driver's eye-height along the major-street approach is assumed to be 3.5 feet above the cross-street pavement.

Stopping sight distance is considered the minimum requirement to ensure safe operation of the site access. This distance allows the driver of a vehicle traveling on the major-street to react to a turning vehicle or other object in the roadway and come to a complete stop to avoid a collision. To ensure safe operation of a site access, the extent of available intersection sight distance must at least equal the minimum stopping sight distance.

Based on an assumed statutory speed of 20 mph for an unpaved gravel roadway in a residential area, the minimum recommended intersection sight distance at the site access location is 225 feet, while the required minimum stopping sight distance to ensure safe operation of the access is 115 feet. Sight distances at the access location were measured to be in excess of 350 feet to the north, and approximately 120 feet to the south (limited by vegetation and vertical curve). If vegetation is cleared, sight distance can be improved to approximately 140 feet to the south.

Provided that the development maintains the minimum acceptable intersection sight distance triangles, including the removal and/or proper maintenance of obstructing roadside vegetation along Highland Drive W, adequate sight distance can be provided to allow safe operation of the site access intersection. Thus, no other sight distance mitigation is necessary or recommended.

## Operational Analysis

A capacity and delay analysis were conducted for each of the study intersections per the unsignalized intersection analysis methodologies in the Highway Capacity Manual (HCM) ${ }^{6}$. Intersections are generally evaluated based on the average control delay experienced by vehicles and are assigned a grade according to their operation. The level of service (LOS) of an intersection can range from LOS A, which indicates very little or no delay experienced by vehicles, to LOS F, which indicates a high degree of congestion and delay. The volume-to-capacity ( $\mathrm{v} / \mathrm{c}$ ) ratio is a measure that compares the traffic volumes (demand) against the available capacity of an intersection.

## Performance Standards

The study intersection of OR-131 at Highland Drive W is under the jurisdiction of ODOT. The applicable minimum operation standard for this facility is established under the Oregon Highway Plan ${ }^{7}$ and is based on the v/c ratio of the intersection. According to the Oregon Highway Plan, OR-131 is a district route located outside any urban growth boundaries and within an unincorporated community and has a maximum allowable v/c ratio of 0.80. The above-mentioned intersection along OR-131 was analyzed according to this standard.

[^9]
## Delay \& Capacity Analysis

The LOS, delay, and $v / c$ results of the capacity analysis are shown in Table 4 for the evening peak period. Detailed calculations as well as tables showing the relationship between delay and LOS are included in the appendix to this report.

Table 4: Capacity Analysis Summary

|  |  |  | ning Peak |  |
| :---: | :---: | :---: | :---: | :---: |
|  |  | LOS | Delay (s) | $\mathrm{v} / \mathrm{c}$ |
| 1. OR-131 at Highland Drive W |  |  |  |  |
|  | 2021 Existing Conditions | B | 10 | 0.00 |
|  | 2023 Background Conditions | B | 10 | 0.00 |
|  | 2023 Buildout Conditions | B | 11 | 0.05 |
|  | 2. Highland Drive | op (sit |  |  |
|  | 2021 Existing Conditions | - | - | - |
|  | 2023 Background Conditions | - | - | - |
|  | 2023 Buildout Conditions | A | 9 | 0.03 |

V/C, Delay, LOS: For two-way stop, these values are taken from the movement with the worst (highest) delay value. BOLDED results indicate operation above acceptable jurisdictional standards

All study intersections are currently operating acceptably per ODOT standards and are projected to continue operating acceptably though the 2023 buildout year, regardless of the potential increase in site trip generation upon development of the site. No operational mitigation is necessary or recommended at these intersections.

## Conclusions

Based on the most recent five years of crash data, no significant trends or crash patterns were identified at the intersection of OR-131 at Highland Drive W that are indicative of safety concerns. Accordingly, no safety mitigation is recommended per the crash data analysis.

Adequate sight distance is available or can be made available (with proper maintenance or removal of roadside vegetation along Highland Drive W), to ensure safe operation for northbound and southbound approaching vehicles at the site access intersection.

Due to insufficient traffic volumes, preliminary traffic signal warrants are not projected to be met at the unsignalized intersection of OR-131 at Highland Drive W under any of the analysis scenarios. In addition, leftturn lane warrants and right-turn lane warrants are not projected to be met at the study intersection under any of the analysis scenarios.

All study intersections are currently operating acceptably per ODOT standards and are projected to continue operating acceptably through the 2023 buildout year, regardless of the potential increase in site trip generation upon development of the site. No operational mitigation is necessary or recommended at these intersections.

## Appendix

TRIP GENERATION CALCULATIONS

Land Use: SingleFamily Detached Housing<br>Land Use Code: 210<br>Setting/Location General Urban/Suburban<br>Variable: Dwelling Units<br>Variable Value: 60

## AM PEAK HOUR

Trip Equation: $\mathrm{T}=0.71(\mathrm{X})+4.80$

|  | Enter | Exit | Total |
| :---: | :---: | :---: | :---: |
| Directional <br> Distribution | $25 \%$ | $75 \%$ |  |
| Trip Ends | $\mathbf{1 2}$ | $\mathbf{3 5}$ | $\mathbf{4 7}$ |

WEEKDAY
Trip Equation: $\operatorname{Ln}(\mathrm{T})=0.92 \mathrm{Ln}(\mathrm{X})+2.71$

|  | Enter | Exit | Total |
| :---: | :---: | :---: | :---: |
| Directional <br> Distribution | $50 \%$ | $50 \%$ |  |
| Trip Ends | 325 | $\mathbf{3 2 5}$ | $\mathbf{6 5 0}$ |

PM PEAK HOUR
Trip Equation: $\operatorname{Ln}(\mathrm{T})=0.96 \mathrm{Ln}(\mathrm{X})+0.20$

|  | Enter | Exit | Total |
| :---: | :---: | :---: | :---: |
| Directional <br> Distribution | $63 \%$ | $37 \%$ |  |
| Trip Ends | $\mathbf{3 9}$ | $\mathbf{2 3}$ | $\mathbf{6 2}$ |

## SATURDAY

Trip Equation: $\operatorname{Ln}(\mathrm{T})=0.94 \mathrm{Ln}(\mathrm{X})+2.56$

|  | Enter | Exit | Total |
| :---: | :---: | :---: | :---: |
| Directional <br> Distribution | $50 \%$ | $50 \%$ |  |
| Trip Ends | $\mathbf{3 0 4}$ | $\mathbf{3 0 4}$ | $\mathbf{6 0 8}$ |



# Gary's Traffic Data <br> 310 Pitney Lane, Unit 39 <br> J unction City, OR 97448 <br> Fast, Accurate, High Quality Counts 

Weather: Cloudy, showers
53 degrees $F$.
Collected By: G.Mc.

File Name : OCNSD Hwy. 131 @ Highland connection
Site Code : Ocnsd.
Start Date : 6/9/2020
Page No : 1

Groups Printed- Unshifted

|  | HWY 131 From North |  |  |  |  | SOUTH AVE. From East |  |  |  |  | HWY 131 From South |  |  |  |  | CAPES DRIVE From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Thru | Right | Peds | App. Total | Left | Thru | Right | Peds | App. Total | Left | Thru | Right | Peds | App. Total | Left | Thru | Right | Peds | App. Total | Int. Total |
| 03:00 PM | 0 | 9 | 1 | 0 | 10 | 1 | 0 | 0 | 0 | 1 | 3 | 17 | 0 | 0 | 20 | 1 | 0 | 0 | 0 | 1 | 32 |
| 03:15 PM | 0 | 13 | 0 | 0 | 13 | 0 | 0 | 0 | 0 | 0 | 3 | 17 | 0 | 0 | 20 | 1 | 0 | 2 | 0 | 3 | 36 |
| 03:30 PM | 0 | 11 | 0 | 0 | 11 | 1 | 0 | 0 | 0 | 1 | 3 | 8 | 0 | 0 | 11 | 0 | 0 | 1 | 0 | 1 | 24 |
| 03:45 PM | 0 | 11 | 0 | 0 | 11 | 0 | 0 | 0 | 0 | 0 | 0 | 15 | 0 | 0 | 15 | 0 | 0 | 1 | 0 | 1 | 27 |
| Total | 0 | 44 | 1 | 0 | 45 | 2 | 0 | 0 | 0 | 2 | 9 | 57 | 0 | 0 | 66 | 2 | 0 | 4 | 0 | 6 | 119 |


| 04:00 PM | 0 | 17 | 0 | 0 | 17 | 0 | 0 | 0 | 0 | 0 | 0 | 14 | 0 | 0 | 14 | 0 | 0 | 3 | 0 | 3 | 34 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 04:15 PM | 0 | 13 | 0 | 0 | 13 | 0 | 0 | 0 | 0 | 0 | 1 | 19 | 0 | 0 | 20 | 0 | 0 | 1 | 0 | 1 | 34 |
| 04:30 PM | 0 | 12 | 0 | 0 | 12 | 0 | 0 | 0 | 0 | 0 | 1 | 10 | 0 | 0 | 11 | 0 | 0 | 2 | 0 | 2 | 25 |
| 04:45 PM | 0 | 12 | 0 | 0 | 12 | 0 | 0 | 0 | 0 | 0 | 3 | 12 | 0 | 0 | 15 | 1 | 0 | 2 | 0 | 3 | 30 |
| Total | 0 | 54 | 0 | 0 | 54 | 0 | 0 | 0 | 0 | 0 | 5 | 55 | 0 | 0 | 60 | 1 | 0 | 8 | 0 | 9 | 123 |


| $05: 00 ~ P M ~$ | 0 | 11 | 0 | 0 | 11 | 0 | 0 | 0 | 0 | 0 | 0 | 13 | 0 | 0 | 13 | 0 | 0 | 1 | 0 | 1 | 25 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| $05: 15 ~ P M ~$ | 0 | 10 | 0 | 0 | 10 | 0 | 0 | 0 | 0 | 0 | 2 | 18 | 0 | 0 | 20 | 0 | 0 | 1 | 0 | 1 | 31 |
| $05: 30 ~ P M ~$ | 0 | 8 | 0 | 0 | 8 | 0 | 0 | 0 | 0 | 0 | 0 | 17 | 0 | 0 | 17 | 0 | 0 | 0 | 0 | 0 | 25 |
| $05: 45 \mathrm{PM}$ | 0 | 3 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 3 | 19 | 0 | 0 | 22 | 0 | 0 | 1 | 0 | 1 | 26 |
| Total | 0 | 32 | 0 | 0 | 32 | 0 | 0 | 0 | 0 | 0 | 5 | 67 | 0 | 0 | 72 | 0 | 0 | 3 | 0 | 3 | 107 |


| Grand Total | 0 | 130 | 1 | 0 | 131 | 2 | 0 | 0 | 0 | 2 | 19 | 179 | 0 | 0 | 198 | 3 | 0 | 15 | 0 | 18 | 349 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Apprch \% | 0 | 99.2 | 0.8 | 0 |  | 100 | 0 | 0 | 0 |  | 9.6 | 90.4 | 0 | 0 |  | 16.7 | 0 | 83.3 | 0 |  |  |
| Total \% | 0 | 37.2 | 0.3 | 0 | 37.5 | 0.6 | 0 | 0 | 0 | 0.6 | 5.4 | 51.3 | 0 | 0 | 56.7 | 0.9 | 0 | 4.3 | 0 | 5.2 |  |

# Gary's Traffic Data <br> 310 Pitney Lane, Unit 39 <br> J unction City, OR 97448 <br> Fast, Accurate, High Quality Counts 

Weather: Cloudy, showers 53 degrees $F$. Collected By: G.Mc.

File Name : OCNSD Hwy. 131 @ Highland connection
Site Code : Ocnsd.
Start Date : 6/9/2020
Page No : 2


Gary's Traffic Data<br>310 Pitney Lane, Unit 39<br>J unction City, OR 97448<br>Fast, Accurate, High Quality Counts

Weather: Cloudy, showers 53 degrees $F$. Collected By: G.Mc.

File Name : OCNSD Hwy. 131 @ Highland connection
Site Code : Ocnsd.
Start Date : 6/9/2020
Page No : 3


# Gary's Traffic Data <br> 310 Pitney Lane, Unit 39 <br> Junction City, OR 97448 <br> Fast, Accurate, High Quality Counts 

Weather: Cloudy, showers
53 degrees $F$.
Collected By: G.Mc.

File Name : OCNSD Hwy. 131 @ Highland connection
Site Code : Ocnsd.
Start Date : 6/9/2020
Page No : 4

|  | HWY 131 From North |  |  |  |  | SOUTH AVE. From East |  |  |  |  | HWY 131 From South |  |  |  |  | CAPES DRIVE From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Thru | Right | Peds | App. Total | Left | Thru | Right | Peds | App. Total | Left | Thru | Right | Peds | App. Total | Left | Thru | Right | Peds | App. Total | Int. Total |
| Peak Hour Analysis From 03:00 PM to 05:45 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  | 04:00 PM |  |  |  |  | 03:00 PM |  |  |  |  | 05:00 PM |  |  |  |  | 04:00 PM |  |  |  |  |  |
| +0 mins. | 0 | 17 | 0 | 0 | 17 | 1 | 0 | 0 | 0 | 1 | 0 | 13 | 0 | 0 | 13 | 0 | 0 | 3 | 0 | 3 |  |
| +15 mins. | 0 | 13 | 0 | 0 | 13 | 0 | 0 | 0 | 0 | 0 | 2 | 18 | 0 | 0 | 20 | 0 | 0 | 1 | 0 | 1 |  |
| +30 mins. | 0 | 12 | 0 | 0 | 12 | 1 | 0 | 0 | 0 | 1 | 0 | 17 | 0 | 0 | 17 | 0 | 0 | 2 | 0 | 2 |  |
| +45 mins. | 0 | 12 | 0 | 0 | 12 | 0 | 0 | 0 | 0 | 0 | 3 | 19 | 0 | 0 | 22 | 1 | 0 | 2 | 0 | 3 |  |
| Total Volume | 0 | 54 | 0 | 0 | 54 | 2 | 0 | 0 | 0 | 2 | 5 | 67 | 0 | 0 | 72 | 1 | 0 | 8 | 0 | 9 |  |
| \% App. Total | 0 | 100 | 0 | 0 |  | 100 | 0 | 0 | 0 |  | 6.9 | 93.1 | 0 | 0 |  | 11.1 | 0 | 88.9 | 0 |  |  |
| PHF | . 000 | . 794 | . 000 | . 000 | . 794 | . 500 | . 000 | . 000 | . 000 | . 500 | 417 | . 882 | . 000 | . 000 | . 818 | . 250 | . 000 | . 667 | . 000 | . 750 |  |

# Gary's Traffic Data <br> 310 Pitney Lane, Unit 39 <br> Junction City, OR 97448 <br> Fast, Accurate, High Quality Counts 

Weather: Cloudy, showers 53 degrees $F$. Collected By: G.Mc.

File Name : OCNSD Hwy. 131 @ Highland connection Site Code : Ocnsd.
Start Date : 6/9/2020
Page No : 5


| SER\# P R J S w date | milepnt | COUNTY ROADS |  | int-type |  |  |  |  |  | SPCL USE |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| invest eau i coday | St from | erst street | RD CHAR | (MEDIAN) | int-Red | OFFRD | wTHR | CRASH |  | LR QTY | move |  |  |  | A | s |  |  |  |  |  |
| Rd dpt el g nhrtime | intersect | SECond Street | DIRECT | legs | traf- | RNDBT | SURF | Coll |  | OWNER | from |  | PRTC | INJ | G | E | LICNS | ped |  |  |  |
| UNLOC? D C S V L K Lat | Long | LRS | Loctn | (\#LANES) | contl | DRVWY | Light | SVRTY | v\# | TYpe | то | P\# | TYPE | SVRTY | E | $x$ | res | Loc | ERROR | act event | CAuse |



Project: 20082 Second Avalon Heights Subdivision
Intersection: OR-131 at Highland Drive W
Date: 3/25/2021
Scenario: 2023 Buildout Conditions

Speed? $\quad 35 \mathrm{mph}$
$\left.\begin{array}{crc}\text { AM Peak Hour } \\ \text { Left-Turn Volume }\end{array} \quad \begin{array}{c}\text { PM Peak Hour } \\ \text { Left-Turn Volume }\end{array}\right] 19$


Source: Oregon DOT Analysis Procedures Manual 2008
*(Advancing Vol \# of Advancing Through Lanes)+ (Opposing Vol/ \# of Opposing Through Lanes)
Note: The criterion is not met from zero to ten left turn vehicles per hour, but careful consideration should be given to installing a left turn lane due to the increased potential for accidents in the through lanes. While the turn volumes are low, the adverse safety and operational impacts may require installation of a left turn. The final determination will be based on a field study.

Project: 20082 Second Avalon Heights Subdivision
Date: 3/25/2021
Scenario: 2023 Buildout Conditions

Speed? $\quad 35 \mathrm{mph} \quad 56 \mathrm{kmh}$

## AM Peak Hour

Right-Turn Volume Approaching DHV Lane Needed?

PM Peak Hour
Right-Turn Volume 19
Approaching DHV 187
Lane Needed? No


Note: If there is no right turn lane, a shoulder needs to be provided.
If this intersection is in a rural area and is a connection to a public street, a right turn lane is needed.

## Traffic Signal Warrant Analysis

| Project: | 20082 Second Avalon Heights Subdivision |  |  |
| :--- | :---: | :---: | :---: |
| Date: | $3 / 25 / 2021$ |  |  |
| Scenario: | 2023 Buildout Conditions |  |  |
| ajor Street: | OR-131 | Minor Street: | Highland Drive W |
| Number of Lanes: | 1 | Number of Lanes: | 1 | | PM Peak |
| :--- |
| Hour Volumes: |

Warrant Used: 100 percent of standard warrants used
$\overline{\mathrm{x}}$ 70 percent of standard warrants used due to 85th percentile speed in excess of 40 mph or isolated community with population less than 10,000.


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.1 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | F |  | F |  |  | $\uparrow$ |
| Traffic Vol, veh/h | 1 | 1 | 167 | 1 | 1 | 139 |
| Future Vol, veh/h | 1 | 1 | 167 | 1 | 1 | 139 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 74 | 74 | 74 | 74 | 74 | 74 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 0 | 0 | 0 |
| Mvmt Flow | 1 | 1 | 226 | 1 | 1 | 188 |


| Major/Minor | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 417 | 227 | 0 | 0 | 227 | 0 |
| Stage 1 | 227 | - | - | - | - | - |
| Stage 2 | 190 | - | - | - | - | - |
| Critical Hdwy | 6.4 | 6.2 | - | - | 4.1 | - |
| Critical Hdwy Stg 1 | 5.4 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.4 | - | - | - | - | - |
| Follow-up Hdwy | 3.5 | 3.3 | - | - | 2.2 | - |
| Pot Cap-1 Maneuver | 596 | 817 | - | - | 1353 | - |
| Stage 1 | 815 | - | - | - | - | - |
| Stage 2 | 847 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 595 | 817 | - | - | 1353 | - |
| Mov Cap-2 Maneuver | 595 | - | - | - | - | - |
| Stage 1 | 815 | - | - | - | - | - |
| Stage 2 | 846 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 10.2 |  | 0 |  | 0.1 |  |
| HCM LOS | B |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 689 | 1353 | - |
| HCM Lane V/C Ratio |  | - | - | 0.004 | 0.001 | - |
| HCM Control Delay (s) |  | - | - | 10.2 | 7.7 | 0 |
| HCM Lane LOS |  | - | - | B | A | A |
| HCM 95th \%tile Q(veh) |  | - | - | 0 | 0 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.1 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | MF |  | F |  |  | $\uparrow$ |
| Traffic Vol, veh/h | 1 | 1 | 168 | 1 | 1 | 140 |
| Future Vol, veh/h | 1 | 1 | 168 | 1 | 1 | 140 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 74 | 74 | 74 | 74 | 74 | 74 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 0 | 0 | 0 |
| Mvmt Flow | 1 | 1 | 227 | 1 | 1 | 189 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Mr |  | $\uparrow$ |  |  | $\uparrow$ |
| Traffic Vol, veh/h | 11 | 11 | 168 | 19 | 19 | 140 |
| Future Vol, veh/h | 11 | 11 | 168 | 19 | 19 | 140 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 74 | 74 | 74 | 74 | 74 | 74 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 0 | 0 | 0 |
| Mvmt Flow | 15 | 15 | 227 | 26 | 26 | 189 |


| Major/Minor | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 481 | 240 | 0 | 0 | 253 | 0 |
| Stage 1 | 240 | - | - | - | - | - |
| Stage 2 | 241 | - | - | - | - | - |
| Critical Hdwy | 6.4 | 6.2 | - | - | 4.1 | - |
| Critical Hdwy Stg 1 | 5.4 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.4 | - | - | - | - | - |
| Follow-up Hdwy | 3.5 | 3.3 | - | - | 2.2 | - |
| Pot Cap-1 Maneuver | 548 | 804 | - | - | 1324 | - |
| Stage 1 | 805 | - | - | - | - | - |
| Stage 2 | 804 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 536 | 804 | - | - | 1324 | - |
| Mov Cap-2 Maneuver | 536 | - | - | - | - | - |
| Stage 1 | 805 | - | - | - | - | - |
| Stage 2 | 786 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 10.9 |  | 0 |  | 0.9 |  |
| HCM LOS | B |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 643 | 1324 | - |
| HCM Lane V/C Ratio |  | - | - | 0.046 | 0.019 | - |
| HCM Control Delay (s) |  | - | - | 10.9 | 7.8 | 0 |
| HCM Lane LOS |  | - | - | B | A | A |
| HCM 95th \%tile Q(veh) |  | - | - | 0.1 | 0.1 | - |

HCM 6th TWSC
2: Highland Drive W \& Roaring Tide Loop (site access)

| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 3.3 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Mr |  | 1 |  |  | $\uparrow$ |
| Traffic Vol, veh/h | 22 | 1 | 1 | 37 | 1 | 1 |
| Future Vol, veh/h | 22 | 1 | 1 | 37 | 1 | 1 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 24 | 1 | 1 | 40 | 1 | 1 |


| Major/Minor | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 24 | 21 | 0 | 0 | 41 | 0 |
| Stage 1 | 21 | - | - | - | - | - |
| Stage 2 | 3 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | - | - | 2.218 | - |
| Pot Cap-1 Maneuver | 992 | 1056 | - | - | 1568 | - |
| Stage 1 | 1002 | - | - | - | - | - |
| Stage 2 | 1020 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 991 | 1056 | - | - | 1568 | - |
| Mov Cap-2 Maneuver | 991 | - | - | - | - | - |
| Stage 1 | 1002 | - | - | - | - | - |
| Stage 2 | 1019 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 8.7 |  | 0 |  | 3.6 |  |
| HCM LOS | A |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 994 | 1568 | - |
| HCM Lane V/C Ratio |  | - | - | 0.025 | 0.001 | - |
| HCM Control Delay (s) |  | - | - | 8.7 | 7.3 | 0 |
| HCM Lane LOS |  | - | - | A | A | A |
| HCM 95th \%tile Q(veh) |  | - | - | 0.1 | 0 | - |

# NETARTS-OCEANSIDE SANITARY DISTRICT <br> 1755 CAPE MEARES LP. RD. W. <br> TILLAMOOK, OR. 97141 <br> PHONE (503)842-8231 <br> FAX (503)842-3759 

Tillamook Co. Department
of Community Development
1510 Third St., Suite B
Tillamook, OR. 97141
(503) 842-3408

DATE: January 21, 2021
TO: TILLAMOOK COUNTY ONE-STOP PERMIT COUNTER

## RE: SEWER AVAILABILITY

I confirm that sanitary sewer service is available to the following lot(s) within our District:
Tax Lot 1S10 30DC 00200

## Availability letter is void after 12 months from the date of issuance.

According to our records, the legal owner is: Avalon Heights, LLC. Bill Hughes, 41901 Old Hwy. 30 Astoria, OR. 97103.

## Sanitary Sewer service is available to the above Tax Lot, but the District does not guarantee that a stub is provided. However, according to the District As-Built map, a stub should be provided.

If the service lateral is to be installed, all at the property owner's expense, the District will require the following:
(a) Secure a permit for utility work in a public road right-of-way.
(b) Minimum 24-hour notice prior to start of work.
(c) Representative of Sanitary District present to inspect installation of the new service lateral.
(d) Notification of all emergency services if road is closed or blocked during installation of the new main service lateral.
(e) Coordinate work with any effected neighboring property owners, so as to minimize inconvenience if road is closed or blocked.
(f) The use of 4" diameter PVC ASTM-3034 pipe for the new service lateral.
(g) The use of a Romac Sanitary Sewer Saddle. Attached, please find Cut-sheet \#418 from our Design Standards, in regards to service saddle connection to existing sewers.

For the lateral from the house to the new stub, the following applies:

1. District requires that property owner/contractor follow APWA Specifications.
2. District requires a Clean-out on the property line, using Schedule 3034 ASTM Pipe with a screw on cap. The Clean-out shall be permanently identified. Attached, please find Cut-sheet \#416.
3. District requires a protective cover if in driveway or a parking zone.
4. It is the responsibility of the property owner to ensure that a copy of the Sewer Availability letter is given to the Contractor.
5. Inspection and testing of the installation shall be done by the Tillamook County Plumbing Inspector in accordance with County requirements.
6. Contractor is responsible for contacting the Tillamook County Inspector to inspect the service lateral.
7. Contractor is responsible for notifying the District to inspect the service lateral connection prior to backfilling. An Inspection Fee will be billed to the property owner at that time.
8. Contractor is responsible for notifying the District office within 5 working days of the service lateral inspection (that is done by Tillamook County Inspector). Failure to notify the District in the allotted time will result in a $\$ 10.00$ per working day fine on the Contractor.

Failure to notify the District for an inspection of the connection, prior to backfilling, will result in one or all of the following fines and/or fees, per District Ordinances:

- \$500.00 Fine for Illegal Connection to the sanitary sewer system.
- $\$ 10.00$ per working day fine on the Contractor (as stated up above).
- A regular User Fee shall be charged to the account plus an amount equal to the regular User Fee, so that the total amount will be double the current established charge for the type of service provided. This charge shall be effective on the date of connection to the public sewer system and shall continue until such time as the account is brought current.

The current System Development Charge fee of $\$ 9,869.00$ per Single Family Dwelling will be due to the Netarts-Oceanside Sanitary District upon issuance of an approved Building Permit by the Tillamook County Departement of Community Development. The District requires that a copy of the approved building permit be sent to the Netarts-Oceanside Sanitary District.

This letter shall not create a liability on the part of Tillamook County, or by an officer, or employee thereof, for the services described above.


Daniel A. Mello, District Superintendent
cc: Property Owner


1. NON-TRAFFIC AREAS:

CARSON MODEL 910 T-COVER OR EQUAL (GREEN FOR SEWER, GREY FOR STORM).
2. TRAFFIC AREAS, INCLUDING DRIVEWAYS:
$8^{n \prime} \times 4^{\prime \prime}$ CAST IRON FRAME \& COVER, OLYMPIC M1007 OR EQUAL.
 AT ALL TIMES.
place excavated materials ON PLYWOOD

NOTE: PER ORS 92.044(7), CLEANOUT boX must be set 1' minimum clear from any surver monument
$221 / 2^{\cdot}$ BEND


串
PLAIN END PIPE, $8^{\prime \prime}$ MIN. LENGTH

22 1/2' STREET BEND
[4" OR 6" SERVICE
PIPE 3034 P.V.C.,
S.D.R. 35

NOTES:

1. CLEANOUT RISER SHALL BE SAME SIZE AND MATERIAL AS LATERAL PIPE.
2. PROVIDE CASTING FOR CLEANOUTS LOCATED IN DRIVEWAYS.
3. CLEANOUT PIPE SHALL BE LEFT A MINIMUM OF 18" ABOVE EXISTING GRADE UNTIL ALL CURBING IS INSTALLED AND ALL PRIVATE UTILITY TRENCHES ARE BACKFILLED. CLEANOUTS SHALL THEN BE SET FLUSH WITH FINISH GRADE.




PVC HUB (ASTM D-3034 SDR 35). DRIVE INTO CENTER OF RUBBER BOOT AFTER BOOT IS PLACED IN CORE HOLE.

- MOLDED PVC SHOULDER TO ACT AS POSITIVE INSERTION STOP ( 4 " \& 6" TAPS).
\#316 STAINLESS STEEL BAND CLAMP ( $9 / 16^{\prime \prime}$ SERIES 300) TO SECURE UPPER HALF OF OF RUBBER SLEEVE TO THE PVC HUB.

MOLDED RUBBER SLEEVE (ASTM C-443) INCLUDES MOLDED RIBS TO HOLD THE SLEEVE IN PLACE IN MAINLINE PIPE TAP, STYLE TO MATCH MAINLINE PIPE MATERIAL \& DIAMETER.

PVC HUB TO BE SHAPED TO MATCH PIPE I.D. AND SHALL NOT PROTRUDE BEYOND INSIDE DIAMETER OF RUBBER BOOT.


NOTES:

1. SANITARY SEWERS - INSERTA-TEES ALLOWED ON EXISTING PVC OR DUCTILE IRON SEWER MAINS. USE ON OTHER PIPE TYPES IS SUBJECT TO CITY APPROVAL AND ACCEPTABLE PIPE CONDITION. MANUFACTURED TEE-WYE FITTINGS SHALL BE USED ON ALL NEW SEWER MAINLINES.
2. THE TAP SHALL NOT BE MADE EXCEPT IN THE PRESENCE OF A NOSD INSPECTOR; NOR SHALL ANY CONNECTION BE MADE WITHOUT NOSD APPROVAL.
3. CENTERLINE OF TAP SHALL BE ABOVE SPRINGLINE.

22 1/2* STREET BEND (ASTM D-3034 PVC, SDR 35) WHERE SHOWN ON LATERAL DETAILS OR PLAN CALLOUTS.

HOLES SHALL BE DRILLED WITH INSERTA-DRILL GUIDE ANCHORED TO PIPE, OR WITH CORE DRILLING MACHINE, IN STRICT ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.

## INSERTA-TEE "FATBOY" FITING SHALL

BE USED FOR ALL $4^{\prime \prime} \& 6^{n}$ TAPS. IN
ORDER TO ALLOW 95\% MANDREL
IESTING OF MAINLINES.

| LAST REVIION DATE: DEC 2012 | STANDARD |
| :---: | :---: |
| INSERTA-TEE CONNECTION TO EXISTING SEWER |  |
| (NTS) |  |
| NOSD, OR | DETAIL NO. $419$ |

## PUBLIC RECORD REPORT <br> FOR NEW SUBDIVISION OR LAND PARTITION

## THIS REPORT IS ISSUED BY THE ABOVE-NAMED COMPANY ("THE COMPANY") FOR THE EXCLUSIVE

 USE OF THE FOLLOWING CUSTOMER:S\&F Land Services
Phone No.:
Date Prepared: December 19, 2020
Effective Date: December 17, 2020 / 08:00 AM
Charge: $\quad \$ 300.00$
Order No.: 360420009017
Reference:
The information contained in this report is furnished to the Customer by Ticor Title Company of Oregon (the "Company") as an information service based on the records and indices maintained by the Company for the county identified below. This report is not title insurance, is not a preliminary title report for title insurance, and is not a commitment for title insurance. No examination has been made of the Company's records, other than as specifically set forth in this report ("the Report"). Liability for any loss arising from errors and/or omissions is limited to the lesser of the fee paid or the actual loss to the Customer, and the Company will have no greater liability by reason of this report. This report is subject to the Definitions, Conditions and Stipulations contained in it.

## REPORT

A. The Land referred to in this report is located in the County of Tillamook, State of Oregon, and is described as follows:

As fully set forth on Exhibit "A" attached hereto and by this reference made a part hereof.
B. As of the Effective Date, the tax account and map references pertinent to the Land are as follows:

As fully set forth on Exhibit "B" attached hereto and by this reference made a part hereof.
C. As of the Effective Date and according to the Public Records, we find title to the land apparently vested in:

As fully set forth on Exhibit " C " attached hereto and by this reference made a part hereof.
D. As of the Effective Date and according to the Public Records, the Land is subject to the following liens and encumbrances, which are not necessarily shown in the order of priority:

As fully set forth on Exhibit "D" attached hereto and by this reference made a part hereof.

Ticor Title Company of Oregon
Public Record Report for New Subdivision or Land Partition
Order No. 360420009017

## EXHIBIT "A"

## (Land Description)

The Southwest quarter of the Southeast quarter of Section 30, Township 1 South, Range 10 West of the Willamette Meridian, in the County of Tillamook, State of Oregon.

EXCEPTING THEREFROM the Plat of Avalon, and the Plat of First Addition to Avalon Heights.
ALSO EXCEPTING THEREFROM that tract conveyed to Carlton Nursery Company, Incorporated by Deed recorded June 29, 1953 in Book 139, page 130, Tillamook County Records.

Ticor Title Company of Oregon
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EXHIBIT "B"

## (Tax Account and Map)

APN/Parcel ID(s) 179176 as well as Tax/Map ID(s) 1S1030DC00200

Ticor Title Company of Oregon
Public Record Report for New Subdivision or Land Partition
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## EXHIBIT "C"

 (Vesting)Avalon Heights LLC, an Oregon limited liability company, which acquired title as Avalon Heights, LLC, an Oregon limited liability company

Ticor Title Company of Oregon
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EXHIBIT "D"

## (Liens and Encumbrances)

1. Regulations, levies, liens, assessments, rights of way and easements of Netarts-Oceanside Sanitary District.
2. Regulations, levies, liens, assessments, rights of way and easements of Netarts Water District.
3. Rights of the public to any portion of the Land lying within the area commonly known as streets, roads, and highways.
4. Easement(s) for the purpose(s) shown below and rights incidental thereto, as granted in a document:

Granted to: John Aschim and Henry Morris
Purpose: Water pipeline
Recording Date: September 11, 1909
Recording No: Book 10, page 410
Affects: Reference is hereby made to said document for full particulars
5. Easement(s) for the purpose(s) shown below and rights incidental thereto, as granted in a document:

Granted to: $\quad$ Netarts Water District
Purpose: Public utilities
Recording Date: November 19, 1973
Recording No: Book 234, page 509
Affects: $\quad$ Reference is hereby made to said document for full particulars
6. Easement(s) for the purpose(s) shown below and rights incidental thereto, as granted in a document:

Granted to: Oceanside Water District
Purpose: Public utilities
Recording Date: July 14, 1983
Recording No: Book 288, page 70
Affects: $\quad$ Reference is hereby made to said document for full particulars
7. Easement(s) for the purpose(s) shown below and rights incidental thereto, as granted in a document:

Granted to: $\quad$ Netarts Water District
Purpose: Public utilities
Recording Date: June 20, 2012
Recording No: 2012-003343
Affects: Reference is hereby made to said document for full particulars

Ticor Title Company of Oregon
Public Record Report for New Subdivision or Land Partition
Order No. 360420009017
EXHIBIT "D"

## (Liens and Encumbrances)

(continued)
8. A deed of trust to secure an indebtedness in the amount shown below,

| Amount: | $\$ 195,000.00$ |
| :--- | :--- |
| Dated: | August 22, 2017 |
| Trustor/Grantor: | Avalon Heights, LLC |
| Trustee: | Ticor Title Insurance Company |
| Beneficiary: | Wauna Federal Credit Union |
| Recording Date: | September 1,2017 |
| Recording No.: | $2017-05254$ |

An agreement to modify the terms and provisions of said deed of trust as therein provided
Executed by: Avalon Heights, LLC and Wauna Credit Union (formerly known as Wauna Federal Credit Union)
Recording Date: $\quad$ November 5, 2018
Recording No: 2018-06592
An agreement to modify the terms and provisions of said deed of trust as therein provided
Executed by: Avalon Heights, LLC and Wauna Credit Union (formerly known as Wauna Federal Credit Union)
Recording Date: July 29, 2019
Recording No: 2019-04407
An agreement to modify the terms and provisions of said deed of trust as therein provided
Executed by: Avalon Heights, LLC and Wauna Credit Union (formerly known as Wauna Federal Credit Union)
Recording Date: July 21, 2020
Recording No: 2020-04531
***END OF EXCEPTIONS***

Note: Property taxes for the fiscal year shown below are paid in full.

| Fiscal Year: | $2020-2021$ |
| :--- | :--- |
| Amount: | $\$ 3,337.42$ |
| Levy Code: | 0935 |
| Account No.: | 179176 |
| Map No.: | 1S1030DC00200 |

Ticor Title Company of Oregon
Public Record Report for New Subdivision or Land Partition
Order No. 360420009017
EXHIBIT "D"

## (Liens and Encumbrances)

(continued)

## BOUNDARY DEEDS:

First Addition to Avalon Heights, Plat Book 3, Page 37
Avalon Heights, Plat Book 3, Page 12
Property Line Adjustment Deed, Document No. 2017-007247
Warranty Deed, Document No. 2011-005156
Quitclaim Deed, Document No. 2019-007699

## DEFINITIONS, CONDITIONS AND STIPULATIONS

1. Definitions. The following terms have the stated meaning when used in this report:
(a) "Customer": The person or persons named or shown as the addressee of this report.
(b) "Effective Date": The effective date stated in this report.
(c) "Land": The land specifically described in this report and improvements affixed thereto which by law constitute real property.
(d) "Public Records": Those records which by the laws of the state of Oregon impart constructive notice of matters relating to the Land.
2. Liability of Company.
(a) This is not a commitment to issue title insurance and does not constitute a policy of title insurance.
(b) The liability of the Company for errors or omissions in this public record report is limited to the amount of the charge paid by the Customer, provided, however, that the Company has no liability in the event of no actual loss to the Customer.
(c) No costs (including without limitation attorney fees and other expenses) of defense, or prosecution of any action, is afforded to the Customer.
(d) In any event, the Company assumes no liability for loss or damage by reason of the following:
(1) Taxes or assessments which are not shown as existing liens by the records of any taxing authority that levies taxes or assessments on real property or by the Public Records.
(2) Any facts, rights, interests or claims which are not shown by the Public Records but which could be ascertained by an inspection of the land or by making inquiry of persons in possession thereof.
(3) Easements, liens or encumbrances, or claims thereof, which are not shown by the Public Records.
(4) Discrepancies, encroachments, shortage in area, conflicts in boundary lines or any other facts which a survey would disclose.
(5) (i) Unpatented mining claims; (ii) reservations or exceptions in patents or in Acts authorizing the issuance thereof; (iii) water rights or claims or title to water.
(6) Any right, title, interest, estate or easement in land beyond the lines of the area specifically described or referred to in this report, or in abutting streets, roads, avenues, alleys, lanes, ways or waterways.
(7) Any law, ordinance or governmental regulation (including but not limited to building and zoning laws, ordinances or regulations) restricting, regulating, prohibiting or relating to (i) the occupancy, use or enjoyment of the land; (ii) the character, dimensions or location of any improvement now or hereafter erected on the land; (iii) a separation in ownership or a change in the dimensions or area of the land or any parcel of which the land is or was a part; or (iv) environmental protection, or the effect of any violation of these laws, ordinances or governmental regulations, except to the extent that a notice of the enforcement thereof or a notice of a defect, lien or encumbrance resulting from a violation or alleged violation affecting the land has been recorded in the Public Records at the effective date hereof.
(8) Any governmental police power not excluded by $2(\mathrm{~d})(7)$ above, except to the extent that notice of the exercise thereof or a notice of a defect, lien or encumbrance resulting from a violation or alleged violation affecting the land has been recorded in the Public Records at the effective date hereof.
(9) Defects, liens, encumbrances, adverse claims or other matters created, suffered, assumed, agreed to or actually known by the Customer.
3. Report Entire Contract. Any right or action or right of action that the Customer may have or may bring against the Company arising out of the subject matter of this report must be based on the provisions of this report. No provision or condition of this report can be waived or changed except by a writing signed by an authorized officer of the Company. By accepting this form report, the Customer acknowledges and agrees that the Customer has elected to utilize this form of public record report and accepts the limitation of liability of the Company as set forth herein.
4. Charge. The charge for this report does not include supplemental reports, updates or other additional services of the Company.

Ticor Title Company of Oregon
Public Record Report for New Subdivision or Land Partition
Order No. 360420009017

## LIMITATIONS OF LIABILITY

"CUSTOMER" REFERS TO THE RECIPIENT OF THIS REPORT.
CUSTOMER EXPRESSLY AGREES AND ACKNOWLEDGES THAT IT IS EXTREMELY DIFFICULT, IF NOT IMPOSSIBLE, TO DETERMINE THE EXTENT OF LOSS WHICH COULD ARISE FROM ERRORS OR OMISSIONS IN, OR THE COMPANY'S NEGLIGENCE IN PRODUCING, THE REQUESTED REPORT, HEREIN "THE REPORT." CUSTOMER RECOGNIZES THAT THE FEE CHARGED IS NOMINAL IN RELATION TO THE POTENTIAL LIABILITY WHICH COULD ARISE FROM SUCH ERRORS OR OMISSIONS OR NEGLIGENCE. THEREFORE, CUSTOMER UNDERSTANDS THAT THE COMPANY IS NOT WILLING TO PROCEED IN THE PREPARATION AND ISSUANCE OF THE REPORT UNLESS THE COMPANY'S LIABILITY IS STRICTLY LIMITED. CUSTOMER AGREES WITH THE PROPRIETY OF SUCH LIMITATION AND AGREES TO BE BOUND BY ITS TERMS
THE LIMITATIONS ARE AS FOLLOWS AND THE LIMITATIONS WILL SURVIVE THE CONTRACT:
ONLY MATTERS IDENTIFIED IN THIS REPORT AS THE SUBJECT OF THE REPORT ARE WITHIN ITS SCOPE. ALL OTHER MATTERS ARE OUTSIDE THE SCOPE OF THE REPORT.

CUSTOMER AGREES, AS PART OF THE CONSIDERATION FOR THE ISSUANCE OF THE REPORT AND TO THE FULLEST EXTENT PERMITTED BY LAW, TO LIMIT THE LIABILITY OF THE COMPANY, ITS LICENSORS, AGENTS, SUPPLIERS, RESELLERS, SERVICE PROVIDERS, CONTENT PROVIDERS AND ALL OTHER SUBSCRIBERS OR SUPPLIERS, SUBSIDIARIES, AFFILIATES, EMPLOYEES, AND SUBCONTRACTORS FOR ANY AND ALL CLAIMS, LIABILITIES, CAUSES OF ACTION, LOSSES, COSTS, DAMAGES AND EXPENSES OF ANY NATURE WHATSOEVER, INCLUDING ATTORNEY'S FEES, HOWEVER ALLEGED OR ARISING, INCLUDING BUT NOT LIMITED TO THOSE ARISING FROM BREACH OF CONTRACT, NEGLIGENCE, THE COMPANY'S OWN FAULT AND/OR NEGLIGENCE, ERRORS, OMISSIONS, STRICT LIABILITY, BREACH OF WARRANTY, EQUITY, THE COMMON LAW, STATUTE OR ANY OTHER THEORY OF RECOVERY, OR FROM ANY PERSON'S USE, MISUSE, OR INABILITY TO USE THE REPORT OR ANY OF THE MATERIALS CONTAINED THEREIN OR PRODUCED, SO THAT THE TOTAL AGGREGATE LIABILITY OF THE COMPANY AND ITS AGENTS, SUBSIDIARIES, AFFILIATES, EMPLOYEES, AND SUBCONTRACTORS SHALL NOT IN ANY EVENT EXCEED THE COMPANY'S TOTAL FEE FOR THE REPORT.

CUSTOMER AGREES THAT THE FOREGOING LIMITATION ON LIABILITY IS A TERM MATERIAL TO THE PRICE THE CUSTOMER IS PAYING, WHICH PRICE IS LOWER THAN WOULD OTHERWISE BE OFFERED TO THE CUSTOMER WITHOUT SAID TERM. CUSTOMER RECOGNIZES THAT THE COMPANY WOULD NOT ISSUE THE REPORT BUT FOR THIS CUSTOMER AGREEMENT, AS PART OF THE CONSIDERATION GIVEN FOR THE REPORT, TO THE FOREGOING LIMITATION OF LIABILITY AND THAT ANY SUCH LIABILITY IS CONDITIONED AND PREDICATED UPON THE FULL AND TIMELY PAYMENT OF THE COMPANY'S INVOICE FOR THE REPORT.
THE REPORT IS LIMITED IN SCOPE AND IS NOT AN ABSTRACT OF TITLE, TITLE OPINION, PRELIMINARY TITLE REPORT, TITLE REPORT, COMMITMENT TO ISSUE TITLE INSURANCE, OR A TITLE POLICY, AND SHOULD NOT BE RELIED UPON AS SUCH. THE REPORT DOES NOT PROVIDE OR OFFER ANY TITLE INSURANCE, LIABILITY COVERAGE OR ERRORS AND OMISSIONS COVERAGE. THE REPORT IS NOT TO BE RELIED UPON AS A REPRESENTATION OF THE STATUS OF TITLE TO THE PROPERTY. THE COMPANY MAKES NO REPRESENTATIONS AS TO THE REPORT'S ACCURACY, DISCLAIMS ANY WARRANTY AS TO THE REPORT, ASSUMES NO DUTIES TO CUSTOMER, DOES NOT INTEND FOR CUSTOMER TO RELY ON THE REPORT, AND ASSUMES NO LIABILITY FOR ANY LOSS OCCURRING BY REASON OF RELIANCE ON THE REPORT OR OTHERWISE.

Ticor Title Company of Oregon
Public Record Report for New Subdivision or Land Partition
Order No. 360420009017

IF CUSTOMER (A) HAS OR WILL HAVE AN INSURABLE INTEREST IN THE SUBJECT REAL PROPERTY, (B) DOES NOT WISH TO LIMIT LIABILITY AS STATED HEREIN AND (C) DESIRES THAT ADDITIONAL LIABILITY BE ASSUMED BY THE COMPANY, THEN CUSTOMER MAY REQUEST AND PURCHASE A POLICY OF TITLE INSURANCE, A BINDER, OR A COMMITMENT TO ISSUE A POLICY OF TITLE INSURANCE. NO ASSURANCE IS GIVEN AS TO THE INSURABILITY OF THE TITLE OR STATUS OF TITLE. CUSTOMER EXPRESSLY AGREES AND ACKNOWLEDGES IT HAS AN INDEPENDENT DUTY TO ENSURE AND/OR RESEARCH THE ACCURACY OF ANY INFORMATION OBTAINED FROM THE COMPANY OR ANY PRODUCT OR SERVICE PURCHASED.

NO THIRD PARTY IS PERMITTED TO USE OR RELY UPON THE INFORMATION SET FORTH IN THE REPORT, AND NO LIABILITY TO ANY THIRD PARTY IS UNDERTAKEN BY THE COMPANY.
CUSTOMER AGREES THAT, TO THE FULLEST EXTENT PERMITTED BY LAW, IN NO EVENT WILL THE COMPANY, ITS LICENSORS, AGENTS, SUPPLIERS, RESELLERS, SERVICE PROVIDERS, CONTENT PROVIDERS, AND ALL OTHER SUBSCRIBERS OR SUPPLIERS, SUBSIDIARIES, AFFILIATES, EMPLOYEES AND SUBCONTRACTORS BE LIABLE FOR CONSEQUENTIAL, INCIDENTAL, INDIRECT, PUNITIVE, EXEMPLARY, OR SPECIAL DAMAGES, OR LOSS OF PROFITS, REVENUE, INCOME, SAVINGS, DATA, BUSINESS, OPPORTUNITY, OR GOODWILL, PAIN AND SUFFERING, EMOTIONAL DISTRESS, NON-OPERATION OR INCREASED EXPENSE OF OPERATION, BUSINESS INTERRUPTION OR DELAY, COST OF CAPITAL, OR COST OF REPLACEMENT PRODUCTS OR SERVICES, REGARDLESS OF WHETHER SUCH LIABILITY IS BASED ON BREACH OF CONTRACT, TORT, NEGLIGENCE, THE COMPANY'S OWN FAULT AND/OR NEGLIGENCE, STRICT LIABILITY, BREACH OF WARRANTIES, FAILURE OF ESSENTIAL PURPOSE, OR OTHERWISE AND WHETHER CAUSED BY NEGLIGENCE, ERRORS, OMISSIONS, STRICT LIABILITY, BREACH OF CONTRACT, BREACH OF WARRANTY, THE COMPANY'S OWN FAULT AND/OR NEGLIGENCE OR ANY OTHER CAUSE WHATSOEVER, AND EVEN IF THE COMPANY HAS BEEN ADVISED OF THE LIKELIHOOD OF SUCH DAMAGES OR KNEW OR SHOULD HAVE KNOWN OF THE POSSIBILITY FOR SUCH DAMAGES.

END OF THE LIMITATIONS OF LIABILITY

Public Record Report for New Subdivision or Partition
(Ver. 20161024)

Reliable Engineering Solutions

TO:
DATE: 04-02-2021
Attn: Melissa Jenk
Tillamook County Department of Community Development - Planning
PROJECT NO. / NAME: E20-036 Second Avalon Heights Subdivision Land Use Package Submittal

## ENCLOSED ARE ONE COPY (unless otherwise noted) OF THE FOLLOWING:

## Description

Signed Application
Application Narrative
Supplemental Narrative, Section 070
Land Use Plans (with Preliminary Plat) -15 Copies
Preliminary Stormwater Report
Geohazard Report
Transportation Impact Study
Road Section Analysis
Netarts Water District Water Availability Letter
Netarts-Oceanside Sewer District Availability Letter
Title Report
Letter From DSL
ODOT Review of Draft TIS
Check for Fees

Please let us now if you need additional copies or if we can assist in any way in your processing and review. A link to the electronic copies has been emailed to you. We look forward to working with you on this project.

THESE ARE TRANSMITTED:

X FOR YOUR REVIEW
$\square$ FOR YOUR SIGNATURE
$\square$ FOR YOUR FILES
OTHER


[^0]:    ${ }^{1}$ Oregon Department of Geology and Mineral Industries. Oregon HazVu: Statewide Geohazards Viewer. https://gis.dogami.oregon.gov/maps/hazvu/
    ${ }^{2}$ Oregon Department of Geology and Mineral Industries. Statewide Landslide Information Database for Oregon (SLIDO). http://gis. dogami.oregon.gov/slido/

[^1]:    ${ }^{3}$ Natural Resource Conservation Service. Web Soil Survey. https://websoilsurvey.sc.egov.usda.gov/App/WebSoilSurvey.aspx

[^2]:    ${ }^{1}$ United States Department of Agriculture, Natural Resources Conservation Service. 2006. Land Resource Regions and Major Land Resource Areas of the United States, the Caribbean, and the Pacific Basin. U.S. Department of Agriculture Handbook 296 ${ }^{2}$ Schlicker, H.G., Deacon, R.J., Beaulieu, J.D. and G.W. Olcott, 1972, Engineering Hazard Map of the Nehalem Quadrangle, Oregon, Scale 1:62,500 in: Environmental Geology of the Coastal Region of Tillamook and Clatsop Counties, Oregon, Oregon Department of Geology and Mineral Industries, Bulletin 74.
    ${ }^{3}$ Wells, R. E., Snavely, P. D., MacLeod, N. S., Kelly, M. M., Parker, M. J., Fenton, J. S., and Felger, T. J., 1995, Geologic map of the Tillamook Highlands, northwest Oregon Coast Range; a digital database: Reston, Va., U.S. Geological Survey Open-File Report 95-670, scale 1:48,000.

[^3]:    ${ }^{4}$ Soil Survey Staff, Natural Resources Conservation Service, United States Department of Agriculture. Web Soil Survey. Available online at: https://wvebsoilsurvey.sc.egov.usda.gov/

[^4]:    ${ }^{5}$ National Flood Insurance Program, Federal Emergency Management Agency. 1978. Flood Insurance Rate Map for Tillamook County, Oregon (Unincorporated Areas) Community Panel Number 4101960165 A.
    ${ }^{6}$ Oregon Department of Geology and Mineral Industries. SLIDO: Statewide Landslide Information Layer for Oregon. Web. Retrieved from: https://gis.dogami.oregon.gov/slido/
    ${ }^{7}$ Schlicker, H.G., Deacon, R.J., Beaulieu, J.D. and G.W. Olcott, 1972, Engineering Hazard Map of the Nehalem Quadrangle, Oregon, Scale 1:62,500 in: Environmental Geology of the Coastal Region of Tillamook and Clatsop Counties, Oregon, Oregon Department of Geology and Mineral Industries, Bulletin 74.
    ${ }^{8}$ Oregon Department of Geology and Mineral Industries. Oregon HazVu: Statewide Geohazards Viewer. Web. Retrieved from: https://gis.dogami.oregon.gov/hazvu/
    ${ }^{9}$ Greenwood, W. 2005. Preliminary Geologic Report for a Proposed 85 Lot Subdivision. Environmental Management Systems, Inc. Milwaukie, OR
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[^5]:    ${ }^{1}$ Oregon Department of Transportation Geographic Information Services. Tillamook County. Map. 2011
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[^6]:    ${ }^{2}$ Institute of Transportation Engineers, Trip Generation Manual, 10 ${ }^{\text {th }}$ Edition, 2017.
    ${ }^{3}$ Oceanside Neighborhood Association, Oceanside Community Plan, 2018.

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